PLANS

# WQI WETLAND PROJECT CAL883416C CALHOUN COUNTY, IOWA

PROJECT LOCATION-

IOWA



	SHEET INDEX
SHEET	DESCRIPTION
A-1	TITLE SHEET
A-2	PROJECT INFORMATION & QUANTITIES
R-1	EXISTING CONDITIONS & REMOVALS
C-1	PROJECT OVERVIEW
C-2	EMBANKMENT PLAN & PROFILE 1
C-3	EMBANKMENT PLAN & PROFILE 2
C-4	ALUMINUM TOE WALL OUTLET STRUCTURE
C-5	DRAWDOWN STRUCTURE PLAN & PROFILE
C-6	WET WELL LIFT STATION PLAN & PROFILE
T-1	TILE 1 PLAN & PROFILE
T-2	TILE 2 PLAN & PROFILE
T-3	TILE 3 PLAN & PROFILE
S-1	DRAWDOWN STRUCTURE DETAILS
S-2	METAL PIPE REQUIREMENTS
S-3	DRAIN TILE DETAILS
S-4	ALUMINUM TOE WALL OUTLET STRUCTURE DETAILS
S-5	ALUMINUM TOE WALL OUTLET STRUCTURE DETAILS
S-6	ALUMINUM TOE WALL OUTLET STRUCTURE DETAILS
S-7	SIGN INSTALLATION
SP-1	SEEDING PLAN

#### NRCS SPECIAL NOTE

NOTE, THIS PROJECT IS IOWA ENGINEER JOB CLASS IV.

#### GENERAL NOTES

WHERE PUBLIC UTILITY FIXTURES ARE SHOWN AS EXISTING ON THE PLANS OR ENCOUNTERED WITHIN THE CONSTRUCTION AREA, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE OWNER OF THESE UTILITIES BY CALLING IOWA ONE CALL (1-800-292-8989) AT LEAST 48 HOURS PRIOR TO THE BEGINNING OF ANY CONSTRUCTION. THE CONTRACTOR SHALL AFFORD ACCESS TO THESE FACILITIES FOR NECESSARY MODIFICATION OF SERVICES. UNDERGROUND TO THESE FACILITIES FOR NECESSARY MODIFICATION OF SERVICES. UNDERGROUND FACILITIES, STRUCTURES AND UTILITIES HAVE BEEN PLOTTED FROM AVAILABLE RECORDS AND SURVEYS, AND THEREFORE THEIR LOCATIONS MUST BE CONSIDERED APPROXIMATE ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THEIR EXISTENCE AND EXACT LOCATION AND TO AVOID DAMAGE THERETO. NO CLAIMS FOR ADDITIONAL COMPENSATION WILL BE ALLOWED TO THE CONTRACTOR FOR ANY INTERFERENCE OR DELAY CAUSED BY SUCH WORK.

#### SURVEY AND DATUM

VERTICAL DATUM: NAVD88 HORIZONTAL DATUM: NAD83/UTM ZONE 15N, IN US SURVEY FEET NOTE, TOPOGRAPHIC SURVEY WAS PROVIDED BY NRCS. A LOCAL BENCHMARK/CONTROL WILL BE SET BY THE ENGINEER PRIOR TO THE START OF CONSTRUCTION AND PROVIDED TO THE CONTRACTOR.







# PROJECT PARTNERS



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I hereby certify that this engineering document was prepared by me or under movidized hare concoll emergingion and that I am 2 data	licensed Professional Engineer under the laws of the State of lowa.	12-2-2024	Meghar M. Funke, PE	My license renewal date is December 31, 2024	Páges or sheets covered by this seal: ALL SHEETS	
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GENERAL PROJECT INFORMATION						
Project ID	Cal883	3416C				
Landowner(s)	Mark Schleisman					
Preliminary or Final Design	Fir	al				
Wetland Drainage Area	154	Acres				
W. Fork Camp Cr. Drainage Area	32,192	Acres				
Time of Concentration	1.75	Hours				
Weighted Runoff Curve Number	91					
Length of Berm	1885	Feet				
Wetland Pool Area	12.5	Acres				
Deep Water Area (>3 ft deep)	3.0	Acres				
Normal Pool Elevation	1196.5	Feet				
Average Pool Depth	2.9	Feet				
Maximum Pool Depth	5.5	Feet				
Pool Storage	32.0	Acre-Feet				
Berm Elevation	1199.0	Feet				
Berm Storage	73.9	Acre-Feet				
Primary Spillway 24-HR Design	5	YR				
Primary Spillway Elevation	1196.5	Feet				
Primary Spillway Weir Width	16	Feet				
5-year Storm Peak Inflow	156	CFS				
5-year Storm Routed Discharge	91	CFS				
5-year Max Surface Elevation	1197.4	Feet				
Auxiliary Spillway 24-HR Design	100	YR				
Auxiliary Spillway Elevation	1197.5	Feet				
Auxiliary Spillway Width	10	Feet				
100-year Storm Peak Inflow	361	CFS				
100-year Storm Routed Discharge	273	CFS				
100-year Max Surf. Elevation	1198.6	Feet				

# ESTIMATED QUANTITIES OF WORK AN

TEM	DESCRIPTION
1	SITE STRIPPING & PREPARATION
2	STRUCTURE & CHANNEL SEEDING
3	BUFFER SEEDING
4	MOBILIZATION
5	DRAIN TILE INVESTIGATION & REMOVAL
6	ALUMINUM TOE WALL OUTLET STRUCTURE
7	EXCAVATION, GENERAL (P)
8	EARTHFILL, GENERAL (P)
9	EARTHFILL, EMBANKMENT (P)
10	EARTHFILL, CORE TRENCH (P)
11	TOPSOILING (P)
12	6" DRAIN TILE, PERF. POLYETHYLENE
13	12" DRAIN TILE, PERF. POLYETHYLENE
14	4" TOE DRAIN TILE, PERF. POLYETHYLENE
15	6" DRAIN TILE OUTLET
16	12" DRAIN TILE OUTLET
17	18" CORRUGATED METAL PIPE
18	DRAWDOWN STRUCTURE
19	RISER INLET STRUCTURE
20	STOP LOG STORAGE STRUCTURE
21	WET WELL & PUMP SYSTEM
22	ROCK RIPRAP (IADOT CLASS E)
23	EROSION STONE
24	CROP DAMAGE

# EARTHWORK BALANCE SUMMARY

	EARTHV	VORK BALANC
ltem	Cubic Yards	
Excavation, General	53800	Total excavation channels, core tr and any other mi Anticipated Borro
Earthfill, General	31500	Total earthfill req and additional fill 20% Shrinkage I
Earthfill, Embankment	13400	Earthfill necessa 20% Shrinkage F
Earthfill Core Trench.	8500	Earthfill necessa
Topsoil	11400	Total stripped top minimally cover of Remaining site to Does not include topsoiling is incid

<u>ND LIST OF SPE</u>		
	UNIT QUANTITY	<u>UNIT</u>
IA CS-001	1	LS
IA CS-006	3.0	AC
IA CS-006	17.6	AC
IA CS-008	1	LS
IA CS-009	1	LS
IA CS-081	1	LS
IA CS-021	53,800	CY
IA CS-023	31,500	CY
IA CS-023	13,400	CY
IA CS-023	8,500	CY
IA CS-026	11,400	CY
IA CS-046	200	LF
IA CS-046	641	LF
IA CS-046	1,855	LF
IA CS-051	20	LF
IA CS-051	40	LF
IA CS-051	212	LF
IA CS-051	1	LS
IA CS-051	1	LS
IA CS-051	1	LS
	1	LS
IA CS-061	653	TON
IA CS-061	60	TON
IA CS-001		AC

E SUMMARY Comment

n required for finish grading of the pool area, rench, sediment basins, auxiliary spillway, borrow hiscellaneous excavation required for the project. row = 0 CY. No Shrinkage Factor.

equired for submerged berms, adjacent fill areas, ill to be placed over tiles to provide adequate cover. Factor

ary to construct the embankment.

Factor

ary to fill core trench. 20% Shrinkage Factor

opsoil to be respread. Use stripped topsoil to embankment, tiles, and other disturbances. topsoil to be used for infills on north side of project. e excess topsoiling for borrow areas. Borrow area dental. No Shrinkage Factor

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#### **FABRICATION NOTES**

- WHEN SEVERAL DIFFERENT COATINGS OR CORRUGATIONS ARE CHECKED IN THE COLUMN BOXES, EACH TYPE IS ACCEPTABLE, BUT ONLY ONE TYPE SHALL BE USED IN EACH INSTALLATION.
- COUPLING BANDS PER DETAILS "A", "B" AND "C" SHALL HAVE THE SAME CORRUGATION REQUIREMENT AND THE SAME COATING AS THE DESIGNATED PIPE.
- 3. ALL WELDS AND ALL HEAT AFFECTED AREAS ON COATED STEEL SHALL BE THOROUGHLY CLEANED AND TREATED IN ACCORDANCE WITH ASTM'S.
- 4. ROD SIZE FOR 8" THRU 15" DIAMETER PIPE SHALL BE 3/8" DIAMETER. FOR PIPE LARGER THAN 15" DIAMETER THE ROD SHALL BE 1/2" DIAMETER. DIAMETER OF HOLES IN THE LUGS SHALL BE 1/8" LARGER THAN THE DIAMETER OF THE ROD USED.
- DURING FABRICATION, WHEN ASPHALT COATING IS NOT USED, RIVETED SEAMS SHALL BE CAULKED WITH AN ASPHALT OR TAR BASED MATERIAL MEETING ASTM A849 TO PROVIDE A WATERTIGHT SEAM. ALL CIRCUMFERENTIAL AND LONGITUDINAL SEAMS SHALL BE CAULKED BEFORE RIVETING. THIS SHALL BE ACCOMPLISHED BY APPLYING A UNIFORM BEAD OF THE ASPHALT OR TAR BASED COMPOUND TO THE INNER LAP SURFACE BEFORE RIVETING SUCH THAT WHEN THE RIVETS ARE IN PLACE, ALL VOIDS
- 6. CLOSE RIVETED PIPE SHALL BE FABRICATED SO THAT THE RIVET SPACING IN THE CIRCUMFERENTIAL SEAMS SHALL NOT EXCEED 3 INCHES, EXCEPT THAT 12 RIVETS SHALL BE SUFFICIENT ON 12" DIA. PIPE.

#### **INSTALLATION NOTES**

- THE SLEEVE TYPE NEOPRENE GASKET SIZE SHALL BE 3/8" THICK WITH A MINIMUM WIDTH OF 7" CENTERED ON THE PIPE JOINT AND FASTENED AT ENDS TO FORM A FULL CIRCLE. IN LIEU OF A NEOPRENE GASKET, ASPHALT OR TAR BASED MASTIC MAY BE USED FOR DETAIL "A", "B" AND "C". (SEE
- 2. IN CONNECTING THE PIPE SECTIONS, THE COUPLING BANDS WILL BE CENTERED ON THE PIPE JOINT AND ALIGNED FOR COMPLETE AND TIGHT NESTING OF CORRUGATIONS BETWEEN COUPLING BAND AND EACH PIPE SECTION. THE GAP BETWEEN THE PIPE SECTIONS, PER DETAIL "A", "B" AND "C" SHALL NOT EXCEED 1 1/2". RODS AND LUGS ON COUPLING BANDS WILL BE INSTALLED ACCORDING TO THE DRAWINGS. THE LUGS FOR DETAIL "C" WILL BE LOCATED IN THE PIPE CORRUGATIONS SO THEY DO NOT INTERFERE WITH EACH OTHER WHEN TIGHTENED. THE NUTS ON THE RODS WILL BE TIGHTENED WITHOUT OVER STRESS AND WILL BE RETIGHTENED AT LEAST TWICE AFTER INITIAL INSTALLATION, AT INTERVALS OF APPROXIMATELY 1/2 HOUR. THE FINAL TENSION ON THE RODS SHALL BE DETERMINED BY THE ENGINEER. BACKFILLING AROUND THE PIPE, EXCEPT AT COUPLING BANDS, MAY PROCEED DURING THE INTERVALS REQUIRED FOR TIGHTENING BANDS.
- 3. BEFORE COUPLING BANDS ARE INSTALLED ON RIVETED PIPE, THE PIPE SECTIONS THAT ARE TO BE CONNECTED SHALL BE ROTATED SO RIVETS OF PIPE ARE ON THE SIDE OF THE PIPE (SEE DETAIL "A") AND THE INSIDE LAPS ARE POINTED DOWNSTREAM.
- ON BITUMINOUS COATED PIPE, REMOVE EXCESS BITUMINOUS COATING FROM CORRUGATIONS WHERE
- 5. THE ENDS OF THE TWO PIPE SECTIONS AND LAP SEAM WILL BE COATED WITH 1/4" OF ASPHALT OR TAR BASED MASTIC (ASTM A849, TROWEL GRADE) FOR DETAIL "A" AND "B" COUPLING BANDS. THE MASTIC COATED AREAS SHOULD BE KEPT FREE OF ALL DIRT, GRAVEL, AND OTHER FOREIGN MATERIAL UNTIL BANDS ARE IN PLACE AND TIGHTENED. WHEN AIR TEMPERATURE IS 50° F, OR LOWER, HEAT WILL BE APPLIED TO SOFTEN. BUT NOT BURN OR MELT. THE MASTIC.
- FLANGE COUPLING BANDS SHALL BE ALIGNED WITH MATCHING SLOTS, AND NUTS ON THE BOLTS TIGHTENED SECURELY. NEOPRENE GASKET OR MASTIC SHALL BE USED BETWEEN FLANGES, AND NUTS WILL BE RETIGHTENED AFTER
- COUPLING BANDS, PER DETAIL "B" AND "C" SHALL NOT EXTEND PAST THE RE-ROLLED END OF THE PIPE SECTION ONTO THE HELICAL CORRUGATIONS







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- 2. All reinforcing steel bars shall be Grade 60 #4 bars. 3.
- 4.
- 5.
- 6.
- Manus-Bond 64-A, or equal.
- Riprap shall be underlain by nonwoven geotextile bedding.

### MATERIAL NOTES

1. Concrete shall have a minimum compressive strength at 28 days equal to 3500 psi. Aluminum sheets shall be structural plate  $(2 \ 1/2" \times 9" \text{ corrugations}) \ 0.100"$  thickness and shall be Alloy 5052 conforming to ASTM B 209 or AASHTO M21. Aluminum angles shall be Alloy 6061–T6 conforming to ASTM B 308. Bolts shall be 3/4" diameter galvanized carbon steel conforming to ASTM A 307. Seam sealant shall be knife-grade asphalt mastic or polymer adhesive sealant tape,

7. Drainfill aggregate shall meet the standard gradation of ASTM C 33 size 57 or 67. Do not place geotextile between the drainfill and 1/2" weep holes. 8. Riprap shall consist of well-graded rock, minimum size of 4", maximum size of 8".







# 3 3 Weir Crest Geotextile Riprap 1/2" Dia. Weep Holes @ 12" O.C. Max. 12' Note 0 00 (m) Note 2 12 O.C. Riprap 14.4 NOTES:

- 1. Drainfill extends completely around weir.
- 2. Field drill 7/8" holes as required in the aluminum structure. Place steel reinforcement through holes. Wrap rebar as described in Construction Note 4.





#### CONSTRUCTION NOTES

- needed.
- required minimum cover.
- pass through the aluminum sheets.
- Place the concrete floor and cutoff wall in one continuous pour. 5.
- After a minimum of 24 hours, place drain fill 6. 7. Backfill around structure with moist soil. Place backfill in 6 inch layers and tamp to a density equivalent to adjacent required earthfill. Remove dry soil from sides of excavations as backfill is placed so that moist soil is tamped against moist soil. Keep backfill approximately level around all parts of the structure.
- 8.
- 9.

STEEL REINFORCING NOTES:

- 1. Radius of bend for Type 20 bar = 2".
- 2. Alternate splice locations for Mark (3) bars may be used at Contractor's option, with Engineer's approval. Minimum splice length is 1'-9".
- 3. Minimum splice length for Mark (4) bars is 1'-4".





lin. ft.

lb.

STEEL SCHEDULE

Mark	Туре	В	С	Quantity	Length	Total Len
1	20	2'-4"	14'-11"	20	17'-3"	345'
2	Str.	_	_	23	2'-4"	53'–8"
3	Str.	-	_	10	21'-8 <u>1</u> "	218'
4	Str.	—	_	15	19	285'
					Total Length	901'-8"
		Weight	= Total Le	ength x 0.6	68 lb./lin. ft.	602.3

#### SECTION ON CENTERLINE



1. Forms are not required for concrete apron or cutoff wall if excavated soil will stand vertically. Side slopes above the top of the concrete shall be 1.5:1 or flatter. 2. When necessary, the site shall be dewatered in accordance with lowa Construction Specification IA-11, Removal of Water. A sump pump and crushed rock may be

3. Set aluminum structure to grade and place reinforcing bars. Apply asphalt mastic between aluminum sheets before bolting together. Check elevations of the weir, top of floor and bottom of cutoff trench. Check that the reinforcing steel has the

4. Wrap reinforcing bars with three layers of electrical tape, or equal, where they

- Place riprap with geotextile bedding and complete grading and shaping.
- Seed, fertilize and mulch all disturbed areas in accordance with the seeding plan.





# STANDARD DETAIL FOR SIGN INSTALLATION

	1				angheers + planners + land surveyors	
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