

# ATOD Will Call Ramp Replacement

State of Iowa - Department of Administrative Services  
1918 SE Hulsizer Rd., Ankeny, IA 50021

PROJECT NUMBER: 9534.00  
RFB 953400-01  
DATE: 06/22/2026  
DRAWING INDEX:

GENERAL	STRUCTURAL
G0.0 COVER	S1.1 DEMO & FOOTING PLAN S1.2 LAYOUT PLAN & ELEVATIONS S1.0 STRUCTURAL NOTES & DETAILS S1.3 RAMP DETAILS

**STRUCTURAL ENGINEER**

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

My license renewal date is: 12/31/2027

*Eric E. Vermeer* 06/22/2026 Date  
Eric E. Vermeer IA Reg. # P19938

**LICENSED PROFESSIONAL ENGINEER**  
IOWA  
P19938

**ARCHITECT**

I hereby certify that the portion of this technical submission described below was prepared by me or under my direct supervision and responsible charge. I am a duly registered architect under the laws of the State of Iowa.

Melissa S. Goodwin Reg. #08372  
*Melissa S. Goodwin* 06/22/2026 Date issued  
Melissa S. Goodwin IA Reg. # 08372

**LICENSED PROFESSIONAL ARCHITECT**  
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**MATERIALS LEGEND**

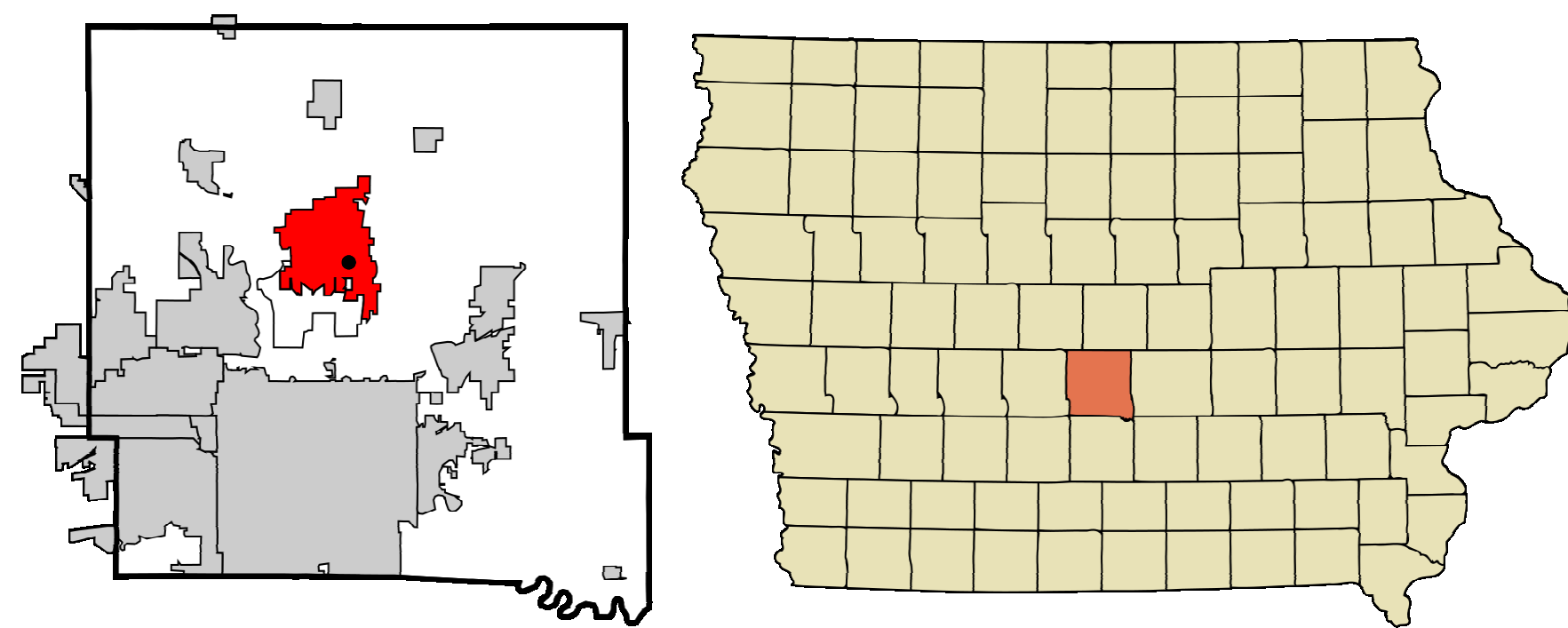
	EARTH		RIGID INSULATION
	CONCRETE		BATT INSULATION
	PLYWOOD		DIMENSION LUMBER
	GYP SUM WALLBOARD		WOOD
	CONCRETE BLOCK		SAND & GRAVEL
			EXPANSION JOINT

**ABBREVIATIONS**

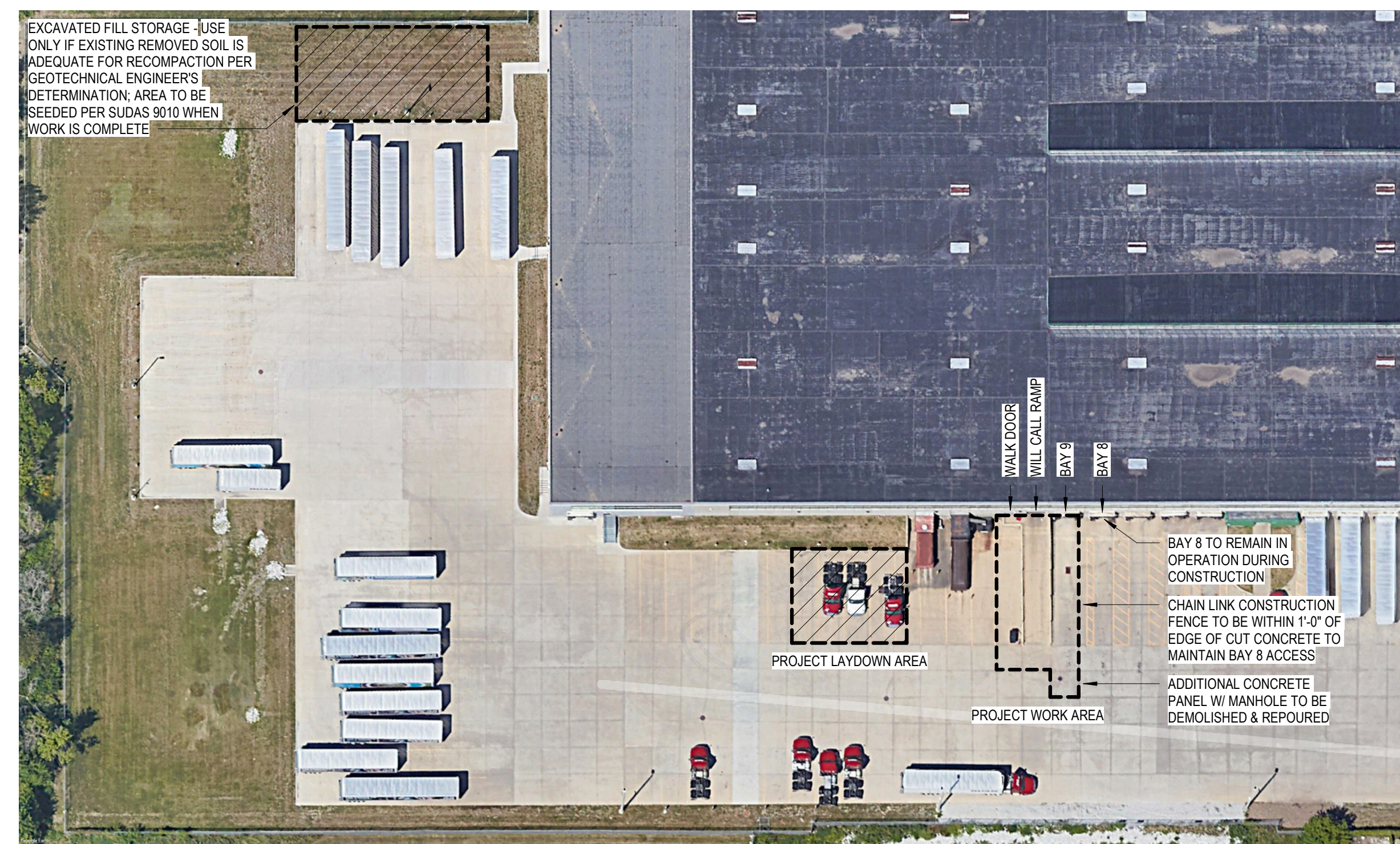
ACT	Acoustical Ceiling Tile
AFF	After Finish Floor
ALUM	Aluminum
ADA &	Americans with Disabilities Act
L	And
∠	Angle
@	At
BCS	Baby Changing Station
BOL	Bollard
CIP	Cast In Place
CBB	Cement Backer Board
CL	Center Line
CLR	Clear
CONC	Concrete
CONF	Conference
CJ	Construction Joint
CONT	Continuous
CLJ	Control Joint
CG	Corner Guard
DIA	Diameter
DS	Downspout
EA	Each
EHD	Electric Hand Dryer
ENC	Electric Water Cooler
EL	Elevation
ELEV	Elevation
ENG	Engineered
EQ	Equal
ETC	Etcetera
EPDM	Ethylene Propylene DieneTerpolymer
EXP	Expansion
EJ	Expansion Joint
EXT	Exterior
EIFS	Exterior Insulation Finish System
FRP	Fiberglass Reinforced Panel
FF	Finish Floor
FE	Fire Extinguisher
FEC	Fire Extinguisher Cabinet
FD	Floor Drain
FT	Foot or Feet
GA	Gage
GT	Glazing Type
GB	Grab Bar
GYP	Gypsum
GYP BD	Gypsum Board
HM	Hollow Metal
HSS	Hollow Structural Sections
IR	Impact Resistant
IT	Information Technology
IMP	Insulated Metal Panel
INSUL	Insulation
INT	Interior
JT	Joint
MFR	Manufacturer
MAX	Maximum
MECH	Mechanical
MEPT	Mechanical/Electrical/Plumbing Technology
MTL	Metal
MIN	Minimum
MIR	Mirror
NIC	Not In Contract
OC	On Center
OHS	Overhead Sectional Door
PT	Paint
PTD	Paper Towel Dispenser
PLAM	Plastic Laminate
PVC	Polyvinyl Chloride
LB	Pound
RCP	Reflected Ceiling Plan
REQD	Required
REV	Reversed
RTU	Roof Top Unit
SNR	Sanitary Napkin Receptacle
SJ	Saw Joint
SIM	Similar
SOS	Soap Dispenser
STL	Steel
STRUCT	Structure
TKSP	Task Strip
ML	Thousands of an Inch
TP	Toilet Paper Dispenser
TOW	Top of Wall
TYP	Typical
VERT	Vertical
w/	With
WD	Wood

**SYMBOLS LEGEND**

	<b>BUILDING SECTION</b> DETAIL NUMBER DRAWING SHEET NUMBER
	<b>WALL SECTION</b> DETAIL NUMBER DRAWING SHEET NUMBER
	<b>DETAIL SECTION/DETAIL CALLOUT</b> DETAIL NUMBER DRAWING SHEET NUMBER
	<b>ELEVATION REFERENCE</b> ELEVATION NUMBER
	<b>DRAWING SHEET NUMBER</b>
	<b>CASEWORK IDENTIFICATION</b> MODEL NUMBER (M DENOTES MODIFICATION) WIDTH DEPTH HEIGHT (INCLUDES COUNTERTOP) EXTRA SHELF MODIFICATION DESCRIPTION
	<b>CEILING IDENTIFICATION</b>
	<b>DOOR IDENTIFICATION</b>
	<b>LEVEL IDENTIFICATION</b> LEVEL ELEVATION
	<b>SPOT ELEVATION</b>
	<b>ROOM IDENTIFICATION</b> ROOM NAME ROOM NUMBER
	<b>WALL IDENTIFICATION</b>
	<b>WINDOW IDENTIFICATION</b>
	<b>NOTE REFERENCE</b>
	<b>STRUCTURAL GRIDLINE</b>
	<b>MATCHLINE</b>
	<b>FLOORING FINISH TRANSITION</b>
	<b>CORNER GUARDS, TYP.</b>



AERIAL VIEW - VICINITY MAP



SITE STAGING PLAN

**GENERAL CONTRACTOR NOTES**

1. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THESE DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK.
2. VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO FABRICATION OF ANY ITEMS.
3. DRAWINGS SHALL BE COORDINATED WITH ELECTRICAL COMPONENTS, NOT SHOWN ON DRAWINGS COORDINATE LOCATION, SIZE AND REINFORCEMENT OF ALL OPENINGS WITH RESPECTIVE TRADES BEFORE FABRICATION. REPORT ANY DISCREPANCIES AND/OR INTERFERENCE PROBLEMS TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
4. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS, NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS, WHEN NO DETAILS ARE PROVIDED, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.
5. THE BUILDING IS NOT STRUCTURALLY STABLE UNTIL ALL CONNECTIONS, FRAMING AND WALLS ARE COMPLETE AND HAVE ACHIEVED FINAL DESIGN STRENGTH. CONTRACTOR IS SOLELY RESPONSIBLE TO MAINTAIN STRUCTURAL STABILITY DURING ERECTION AND CONSTRUCTION. TEMPORARY BRACING SYSTEMS ARE NOT TO BE REMOVED UNTIL STRUCTURAL WORK IS COMPLETE.
6. CONTRACTORS CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
7. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL MEANS, METHODS, TECHNIQUES, SEQUENCING, AND PROCEDURES NECESSARY FOR CONSTRUCTING, OVERSEEING, AND COORDINATING THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND ALL APPLICABLE HEALTH AND SAFETY STANDARDS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SITE SAFETY.

**DESIGN PARAMETERS**

1. THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE ICC INTERNATIONAL BUILDING CODE, 2015 EDITION.
2. RAMP LANDING FROST SLAB DESIGN BASED ON ACCOMMODATING A 26,000 LBS CAPACITY BOXTRUCK.

4. SNOW DESIGN PARAMETERS

GROUND SNOW LOAD (Ps)	30 PSF
FLAT ROOF SNOW LOAD (P <sub>r</sub> )	23 PSF
SNOW EXPOSURE FACTOR (CE)	1.0
SNOW LOAD IMPORTANCE FACTOR (I)	1.0
SNOW LOAD THERMAL FACTOR (CT)	1.0

5. EARTHQUAKE DESIGN PARAMETERS

SEISMIC IMPORTANCE FACTOR (IE)	1.0
S <sub>s</sub>	0.04g
S <sub>1</sub>	0.04g
SEISMIC SITE CLASS	D
S <sub>MS</sub>	0.07g
SHORT-PERIOD SITE COEFFICIENT (F <sub>a</sub> )	1.0
LONG-PERIOD SITE COEFFICIENT (F <sub>v</sub> )	2.4

SEISMIC FORCE-RESISTING SYSTEM

ORDINARY REINFORCED CONCRETE WALLS	A
SEISMIC BASE SHEAR (V) <sub>CL</sub> CLASSROOM ADDITION	0.70K
SEISMIC RESPONSE COEFFICIENT (C <sub>s</sub> ) N-S	0.01
SEISMIC RESPONSE COEFFICIENT (C <sub>s</sub> ) E-W	0.01
RESPONSE MODIFICATION FACTOR (R) N-S	2.0
RESPONSE MODIFICATION FACTOR (R) E-W	2.0
REDUNDANCY FACTOR (μ) N-S	1.0
REDUNDANCY FACTOR (μ) E-W	1.0

6. WIND DESIGN PARAMETERS

BASIC WIND SPEED (V) (1 SECOND GUST)	115 MPH
RISK CATEGORY	C
WIND EXPOSURE CATEGORY	1
INTERNAL PRESSURE COEFFICIENT	+/- 0.18
BASIC DESIGN PRESSURE @ WALL	13.87 (SERVICE)

7. ALLOWABLE FUTURE CONSTRUCTION

VERTICAL	NONE
HORIZONTAL	NONE

8. ASSUMED FUTURE CONSTRUCTION

VERTICAL	NONE
HORIZONTAL	NONE

9. ASSUMED FUTURE CONSTRUCTION

NONE	
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**FOUNDATION PARAMETERS**

SOIL AND BOLT ELEVATIONS OF FOOTINGS HAVE BEEN DETERMINED BASED ON SITE OBSERVATIONS. UNANTICIPATED SOIL CONDITIONS MAY REQUIRE CHANGES. OWNER IS TO EMPLOY A GEOTECHNICAL ENGINEER TO REVIEW SOIL CONDITIONS AT NEW FOOTINGS PRIOR TO CONCRETE PLACEMENT. AS EXCAVATION PROGRESSES, UNANTICIPATED SOIL CONDITIONS MAY REQUIRE CHANGES. CONTACT STRUCTURAL ENGINEER OF RECORD FOR EVALUATION OF THESE CHANGES.

2. ALLOWABLE SOIL BEARING PRESSURE

SPREAD AND ISOLATED FOOTINGS	2000 PSF (ASSUMED)
RETAINING FOOTINGS (AT FOOTING TOE)	2000 PSF (ASSUMED)

3. MINIMUM FOOTING DEPTH FOR FROST PROTECTION

BELOW FINISHED GRADE	42 IN.
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4. DESIGN EARTH PRESSURES:

ACTIVE	35 PCF
PASSIVE	400 PCF
AT REST	60 PCF

6. GEOTECHNICAL ENGINEER IS SOLE JUDGE OF THE SUITABILITY OF UNDERLYING MATERIAL TO SUPPORT FOUNDATIONS. BEARING MATERIAL SHALL BE APPROVED BY GEOTECHNICAL ENGINEER BEFORE FOUNDATION INSTALLATION. DO NOT PROCEED WITHOUT APPROVAL.

**SUBMITTALS**

1. GENERAL CONTRACTOR TO PROVIDE A SHOP DRAWING SUBMITTAL LOG ITEMIZING ALL PROPOSED SUBMITTALS FOR APPROVAL BY STRUCTURAL ENGINEER OF RECORD.

2. ALL SHOP DRAWINGS SHALL BE CHECKED BY THE FABRICATOR AND APPROVED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD. SHOP DRAWINGS REVIEW BY ENGINEER IS LIMITED TO VERIFYING GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. CONTRACTOR IS RESPONSIBLE FOR ANY CHANGES FROM THE CONTRACT DOCUMENTS, DIMENSIONAL ERRORS, COORDINATION ERRORS, OR OMISSIONS IN SHOP DRAWINGS.

3. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS, INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING:

- CONCRETE MIX DESIGNS (5 DAYS BEFORE POUR, MIN.)
- CONCRETE REINFORCEMENT
- EMBEDDED STEEL ITEMS
- CONCRETE CURING
- STEEL GUARDRAILS

4. SHOP DRAWINGS SHALL INCLUDE CONNECTIONS AS WELL AS SIZE, SPACING, AND GRADE OF ALL MEMBERS, PLANS AND ANY DETAILING NECESSARY FOR DETERMINING FIT AND PLACEMENT SHALL ALSO BE INCLUDED.

5. IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF AN ENGINEER REGISTERED IN THE APPROPRIATE STATE, ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND BE SUBJECT TO REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER.

6. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY THE CONTRACTOR, INCLUDING:

- REINFORCING
- CONCRETE FORMWORK

7. ITEMS THAT ARE DESIGNED BY THE CONTRACTOR SHALL BE DESIGNED TO RESIST THE LIVE LOADS INDICATED IN STRUCTURAL NOTES, DEAD LOAD, SELF WEIGHT, ANY ADDITIONAL LOADING INDICATED ON PLANS AND DETAILS, SNOW DRIFT, AND A NET WIND UPLIFT.

8. ITEMS THAT ARE DESIGNED BY THE CONTRACTOR SHALL INCLUDE ANY RELEVANT TECHNICAL LITERATURE FROM MANUFACTURER, ALSO PROVIDE A CERTIFICATION FROM THE MANUFACTURER SHOWING THE PRODUCT IS IN COMPLIANCE WITH ALL APPLICABLE CODES AND STANDARDS.

9. FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF AN ENGINEER REGISTERED IN THE APPROPRIATE STATE AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

10. ALL SHOP DRAWINGS SHALL BE CHECKED BY THE FABRICATOR AND APPROVED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT. WHOSE REVIEW OF THE SHOP DRAWINGS IS LIMITED TO VERIFYING GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. CONTRACTOR IS RESPONSIBLE FOR ANY CHANGES FROM CONTRACT DOCUMENTS, DIMENSIONAL ERRORS, COORDINATION ERRORS, OR OMISSIONS IN SHOP DRAWINGS.

11. VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO FABRICATION OF ANY ITEMS. DRAWINGS SHALL BE COORDINATED WITH MECHANICAL/ELECTRICAL COMPONENTS NOT SHOWN ON DRAWINGS. COORDINATE LOCATION, SIZE, AND REINFORCING OF ALL OPENINGS WITH RESPECTIVE TRADES BEFORE FABRICATION.

12. FIELD VERIFY EXISTING DIMENSIONS, MEMBER SIZES AND ELEVATIONS SHOWN ON THE DRAWINGS PRIOR TO STARTING CONSTRUCTION. ALL DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT.

**CONCRETE NOTES**

1. CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE IBC.

2. WHERE NEW CONCRETE IS PLACED DIRECTLY AGAINST EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE CLEANED AND ROUGHENED TO A MINIMUM OF 1/4" AMPLITUDE. SEE PLAN FOR DOWELING REQUIREMENTS.

3. ALL EXPOSED EDGES OF CONCRETE MEMBERS SHALL BE CHAMFERED 1/2" U.N.O.

4. CONCRETE FORMWORK:

- 4.1 SLEEVES, OPENINGS, CONDUIT, AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO POURING CONCRETE.
- CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD OF THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS TO CENTER.
- 4.2 VERIFY ALL BLOCK OUTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING REQUIREMENTS PRIOR TO POURING.

5. CONCRETE REINFORCEMENT:

- 5.1 DETAILING, FABRICATION, AND PLACEMENT OF REINFORCEMENT SHALL CONFORM TO ACI-318.
- 5.2 ALL REINFORCEMENT TO BE ASTM A618 GRADE 60 U.N.O. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE USING #16 ANNEALED IRON WIRE. REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.

**GENERAL CONTRACTOR NOTES**

- 5.3 ALL CONTINUOUS REINFORCING SHALL BE SPLICED USING CLASS B TENSION SPLICES, U.N.O.
- 5.4 BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS AS SPECIFIED BY THE CRS MANUAL OF STANDARD PRACTICES, REV. 1.
- 5.5 CONTINUE HORIZONTAL WALL BARS THROUGH PLASTERS, COLUMNS, AND INTERSECTING WALLS. REFER TO TYPICAL DETAILS FOR LAYOUTS OF CORNER BARS AND BARS IN SMALL WALL SECTIONS. SLAB BARS SHALL BE HOOKED INTO WALLS OR HOOKED DOWELS SHALL BE PROVIDED TO MATCH SLAB REINFORCING. PROVIDE HOOKED DOWELS FROM FOOTINGS TO MATCH VERTICAL WALL REINFORCING.
- 5.6 ADD TWO DIAGONAL #3 BARS, FOUR FEET LONG, CENTERED, AT EACH CORNER OF FOUNDATION OR SLAB OPENING, U.N.O.
- 5.7 ALL SLAB REINFORCEMENT IS TO BE FULLY CHAIRED AT THE INDICATED ELEVATION PER ACI-318 REQUIREMENTS PRIOR TO CONCRETE PLACEMENT. LIFTING SLAB MAT REINFORCEMENT INTO PLACE DURING CONCRETE PLACEMENT IS NOT PERMITTED.

**COVERAGE FOR REINFORCEMENT:**

6.1 CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
6.2 CONCRETE EXPOSED TO WEATHER OR WEATHER:	2"
#6 THRU #18 BARS	1 1/2"
#5 BAR	

6.3 CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:

SLAB WALLS, JOISTS:	3/4"
#11 BAR AND SMALLER:	
PIERS:	1 1/2"

7. CONCRETE COMPRESSIVE STRENGTHS SHALL BE VERIFIED BY STANDARD 28-DAY CYLINDER TESTS PER ASTM C39 AND SHALL BE AS FOLLOWS:

FOOTINGS:	f <sub>c</sub> = 4,000 PSI
SLABS ON GRADE:	f <sub>c</sub> = 4,000 PSI
WALLS AND PIERS:	f <sub>c</sub> = 4,000 PSI

8. SPECIAL INSPECTOR SHALL BE NOTIFIED IF ANY WATER IS TO BE ADDED IN FIELD.

**CONCRETE FOUNDATIONS:**

8.1 ALL FOUNDATION EXCAVATIONS MUST BE REVIEWED AND APPROVED BY THE GEOTECHNICAL ENGINEER TO BE OF SUITABLE BEARING CAPACITY PRIOR TO PLACEMENT OF CONCRETE.

8.2 ACCURATELY SET AND SECURELY SUPPORT REINFORCING, DOWELS, AND ANCHOR BOLTS PRIOR TO PLACEMENT OF CONCRETE. WET STICKING OR FLOATING OF REINFORCING, DOWELS, AND ANCHOR BOLTS IS NOT ALLOWED.

8.3 PROVIDE CONSTRUCTION JOINTS, INSERTS, SLEEVES, DOWELS, ANCHORS, ETC. AS SHOWN. ITEMS SHALL BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS AND ACCORDING TO USUAL ACCEPTED STANDARDS OF THE TRADE. ANCHOR BOLTS PER ASTM F1554 AS SHOWN.

8.4 ALL FOUNDATION COLD JOINTS SHALL BE KEPT. CONTINUOUS REINFORCING SHALL EXTEND ONE BOLT EQUIVALENT LENGTH PLUS 12".

8.5 REINFORCING SHOWN ON DETAILS IS THE REQUIRED MINIMUM AT CONTRACTOR'S OPTION AND EXPENSE. ADDITIONAL REINFORCING WILL BE PERMITTED TO ASSIST IN EASE OF CONSTRUCTION. ADDITIONAL REINFORCING MUST BE SUBMITTED FOR REVIEW ON SHOP DRAWINGS.

10. PROVIDE MINIMUM 7000 PSI NON-SHRINK, NON-METALLIC GROUT. GROUT SHALL NOT CONTAIN GYPSUM. ENSURE ALL VOIDS ARE FILLED WITH GROUT.

11. NOT USED

12. PROVIDE CONTROL JOINTS AS FOLLOWS, U.N.O.:

- 12.1 SLABS (ON GRID LINE WHERE PRACTICAL) AND SLABS ON GRADE: 10' ± 2'
- 12.2 CONTROL JOINT LAYOUT SHOULD MAINTAIN AN APPROXIMATE ASPECT RATIO OF 1:1

13. EXPOSED SLAB ON GRADE:

- 13.1 ALL SLAB ON GRADE CONTROL JOINTS & COLD JOINTS SHALL BE COORDINATED WITH THE ARCHITECT WHERE THE SLAB IS EXPOSED.
- 13.2 CONTRACTOR SHALL PROVIDE CONTROL JOINTS IN A PRE-CONSTRUCTION MEETING PRIOR TO ANY EXPOSED CONCRETE SLAB ON GRADE POUR. CONTRACTOR SHALL PROVIDE ENGINEER WITH ANTICIPATED EXPOSED SLAB ON GRADE CONTROL AND COLD JOINT LOCATIONS. ENGINEER MUST APPROVE CONTROL AND COLD JOINT LOCATIONS PRIOR TO ANY EXPOSED CONCRETE SLAB POUR.
- 13.3 SLABS SHALL BE SUPPORTED BY REINFORCED ENGINEERED COMPACTED FILL IN THICKNESS AS NOTED ABOVE. SLABS ON GRADE SHALL BE AS INDICATED ON DRAWINGS, WHERE REQUIRED BY GEOTECHNICAL ENGINEER. OVER EXCAVATE AND RECOMPACT USING STRUCTURAL FILL. SEE SOILS REPORT FOR SPECIFIC RECOMMENDATIONS FOR PROTECTIVE SURFACE. PROVIDE NO MORE THAN 15% FLY ASH FOR ALL FLOOR SLABS.
- 13.4 SLAB CONTROL JOINTS SHALL BE 18" WIDE AND 1/4" OF THE SLAB THICKNESS DEEP. CUTTING OPERATIONS SHALL BE FROM 4 TO 12 HOURS AFTER PLACING CONCRETE. MAXIMUM SPACING OF JOINTS SHALL BE 30 TIMES THE SLAB THICKNESS OR AS SHOWN ON THE DRAWINGS. SEE SPECIFICATION FOR REINFORCING CUTTING REQUIREMENTS AND EDGE RAVELING. CONTRACTOR TO SUBMIT SAWN JOINT LAYOUT TO ENGINEER FOR APPROVAL PRIOR TO POURING SLABS.

14. CONCRETE ACCESSORIES:

- 14.1 U.N.O. EXPANSION BOLTS TO BE HLT11K1K1K BOLT, 8. SIMPSON STRONG-BOLT, OR APPROVED EQUIVALENT WITH EQUAL ICC TENSION AND SHEAR VALUES. EXPANSION ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
- 14.2 U.N.O. EPOXY ANCHORS TO BE HLT11K1K1K BOLT, 8. SIMPSON STRONG-BOLT, OR APPROVED EQUIVALENT WITH EQUAL ICC TENSION AND SHEAR VALUES. EPOXY ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.

15. PERMANENTLY EXPOSED PLATES AND GUARDRAILS TO BE HOT-DIP GALVANIZED.

16. EPOXY REPAIR ADHESIVE SHALL CONFORM TO ASTM C815 AND SHALL BE A TWO-COMPONENT, LIQUID EPOXY WITH NON-SAG CONSISTENCY AND A LONG POT LIFE. THE EPOXY ADHESIVE SHALL BE SUITABLE FOR USE IN DRY OR DAMP CONDITIONS. MINIMUM SHEAR STRENGTH SHALL BE 2,000 PSI. MINIMUM TENSILE STRENGTH SHALL BE 4,000 PSI. HOLE SIZES AND INSTALLATION SHALL BE IN STRICT COMPLIANCE WITH THE APPROVED ICC REQUIREMENTS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING PLACEMENT.

17. COLD WEATHER PROTECTION:

- 17.1 DURING COLD WEATHER CONSTRUCTION, CONTRACTOR IS RESPONSIBLE FOR PROTECTING CURING CONCRETE FROM THE RISKS OF ACI 308.
- 17.2 DURING COLD WEATHER CONSTRUCTION, CONTRACTOR IS RESPONSIBLE FOR PROTECTING FOUNDATIONS THAT DO NOT EXTEND TO FROST DEPTH FROM HEAVING, AT CONTRACTOR'S OPTION. BOTTOM OF FOOTINGS MAY BE REINFORCED WITH 20# REINFORCING TO PREVENT FROST PROTECTION METHODS. NO ADDITIONAL REINFORCING WILL BE REQD AT EXTENDED FOUNDATIONS. ALL MITIGATION WORK SHALL BE PART OF THE CONTRACTOR BID AND NOT THE ARCHITECT'S RESPONSIBILITY.
- 18.3 CONTRACTOR TO PROVIDE A FROST STOOP PER TYP. STOOP DETAIL @ ALL EXTERIOR DOORS. SEE ARCH. FOR DOOR LOCATIONS.

**STEEL FRAMING NOTES**

1. STEEL FRAMING WORK SHALL COMPLY WITH CHAPTER 22 OF THE IBC.

2. STEEL FRAMING MATERIALS:

- 2.1 WWT SECTIONS: ASTM A992 Fy=50,000 PSI
- 2.2 TUBULAR STEEL: ASTM A500 GR B Fy=46,000 PSI
- 2.3 HANGER AND ANCHOR BARS: ASTM A36 Fy=36,000 PSI
- 2.4 PLATES (NON-MOMENT CONNECTIONS): ASTM A36 Fy=36,000 PSI
- 2.5 PLATES (MOMENT CONNECTIONS): ASTM A572 Fy=50,000 PSI
- 2.6 WELDING ELECTRODES: E70XX

3. STEEL ERECTION SHALL COMPLY WITH ALL OSHA, STATE, LOCAL, AND UNION REGULATIONS. IN ADDITION, AT THE END OF THE WORK DAY, ALL COLLUMS MUST BE FRAMED WITH BEAMS AND GIRDERS IN ALL DIRECTIONS, OR ADEQUATELY GUYED OR BRACED IN ALL DIRECTIONS. TEMPORARY GUYING/BRACING IS THE RESPONSIBILITY OF THE ERECTOR (DESIGN AND INSTALLATION).

4. DESIGN, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC STEEL CONSTRUCTION MANUAL, AISC 360-10 INCLUDING COMMENTARY, AND THE AISC 360-10.

5. ALL BOLTS FOR STRUCTURAL STEEL CONNECTIONS PER ASTM A325 U.N.O. AND SHALL CONFORM TO THE RCSC SPECIFICATION. BOLTS FOR STEEL JOISTS MAY BE A307. BOLTS AND SHALL BE SNUG-TIGHT U.N.O.

6. WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDED PROCEDURE SPECIFICATION (WPS) AS REQUIRED BY AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER.

7. WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE #1/8" MINIMUM U.N.O. ALL WELDING FOR STRUCTURAL STEEL CONNECTIONS SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH AWS D1.1. PROTECT ALL SURFACES SURROUNDING WELDING OPERATIONS.

8. NOT USED

9. NOT USED

10. NOT USED

11. HOT-DIP GALVANIZED STEEL FOR THE FOLLOWING CONDITIONS:

- 11.1 STEEL ELEMENTS PERMANENTLY EXPOSED TO WEATHER.
- 11.2 STEEL ELEMENTS LOCATED OUTSIDE THE BUILDING THERMAL ENVELOPE.
- 11.3 STEEL ELEMENTS LOCATED IN AREAS WITH HIGH HUMIDITY.
- 11.4 WHERE PROCESS REQUIRING GALVANIZATION ARE PART OF A LARGER ASSEMBLY, THE HOT-DIPPING PROCESS IS TO OCCUR AFTER COMPLETE FABRICATION OF THE ASSEMBLY.

12. CONNECTIONS:

- 12.1 WHERE CONNECTIONS ARE NOT SHOWN, THE CONNECTIONS SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE REQUIREMENTS OF AISC SPECIFICATIONS FOR THE SHEAR CAPACITIES SHOWN BELOW.
- 12.2 ALL CONNECTIONS SHALL SATISFY THE REQUIREMENTS OF AISC AND OSHA.
- 12.3 AT STEEL CONNECTIONS WITH POST INSTALLED ANCHORS, THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THAT THE HOLE DIAMETERS COMPLY WITH THE MANUFACTURER'S RECOMMENDED DRILL BIT DIAMETER.

**SPECIAL INSPECTION SCHEDULE**

1. SPECIAL INSPECTION PROGRAM SHALL CONFORM TO CHAPTER 17 OF THE IBC.

2. THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM THE REQUIRED TESTS AND SPECIAL INSPECTIONS WITH QUALIFICATIONS DESCRIBED PER IBC CHAPTER 17 AND THE PROJECT SPECIFICATIONS.

3. SPECIAL INSPECTION REPORTS SHALL BE FURNISHED TO BUILDING OFFICIAL, OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND CONTRACTOR.

4. SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING THAT THE STRUCTURAL WORK WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, PERFORMED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

5. SPECIAL INSPECTION PROGRAM:

**VERIFICATION AND INSPECTION**

STEEL	FREQ/TASK
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1. INSPECTION TASKS PRIOR TO WELDING:

- A. WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE: PERFORM
- B. MANUFACTURER'S CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE: PERFORM
- C. MATERIAL IDENTIFICATION (TYPE/GRADE): OBSERVE
- D. WELDER IDENTIFICATION SYSTEM: OBSERVE
- E. CONFIGURATION & FINISH OF ACCESS HOLES: OBSERVE

F. FIT UP OF FILED WELDS:

- DIMS (ALIGNMENT, GAPS AT FOOT)
- CLEANLINESS (CONDITION OF STEEL SURFACES)
- TACKLING (TACK WELD QUALITY & LOCATION)

2. INSPECTION TASKS DURING WELDING:

- A. USE OF QUALIFIED WELDERS: OBSERVE
- B. CONTROL & HANDLING OF WELDING CONSUMABLES: OBSERVE
- EXPOSURE CONTROL - PACKAGING
- C. NO WELDING OVER CRACKED TACK WELDS: OBSERVE
- D. ENVIRONMENTAL CONDITIONS: OBSERVE
- WIND SPEED WITHIN LIMITS - PRECIPITATION & TEMP.

E. WPS FOLLOWED:

- SETTINGS ON WELDING EQPT. - TRAVEL SPEED
- SELECTED WELDING MATERIALS - PREHEAT APPLIED
- SHIELDING GAS TYPE/FLOW RATE - PROPER POSITION (F, V, H, OH)
- INTERPASS TEMP. MAINTAINED (MIN/ MAX)

F. WELDING TECHNIQUES:

- INTERPASS & FINAL CLEANING
- EACH PASS WITHIN PROFILE LIMITATIONS
- EACH PASS MEETS QUALITY REQUIREMENTS

3. INSPECTION TASKS AFTER WELDING:

- A. WELDS CLEANED: OBSERVE
- B. WELD LENGTH & LOCATION OF WELDS: PERFORM
- C. WELDS MEET VISUAL ACCEPTANCE CRITERIA: OBSERVE
- CRACK PROHIBITION - WELD SIZE
- WELD / BASE-METAL FUSION - UNDERCUT
- CRATER CROSS SECTION - POROSITY
- WELD PROFILES

D. ARC STRIKES: PERFORM

E. K-AREA: PERFORM

F. BACKING & WELD TABS REMOVED (IF REQ'D.): PERFORM

G. REPAIR ACTIVITIES: PERFORM

H. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER: PERFORM

I. INSPECTION TASKS PRIOR TO BOLTING:

- A. MEASUREMENTS AVAILABLE FOR FASTENER MATERIALS: PERFORM
- B. FASTENERS MARKED IN ACCORDANCE W/ ASTM REQ'TS: OBSERVE
- C. PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH, EXCLUDED THREADS): OBSERVE
- D. PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL: OBSERVE
- E. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION & HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQ'TS: OBSERVE
- F. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED & DOCUMENTED FOR FASTENER ASSEMBLIES & METHODS USED: OBSERVE

G. PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS & OTHER FASTENER COMPONENTS: OBSERVE

INSPECTION TASKS DURING BOLTING:

- A. FASTENER ASSEMBLIES OF SUITABLE CONDITION, PLACED IN ALL HOLES & WASHERS (IF REQ'D) ARE POSITIONED AS REQ'D: OBSERVE
- B. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRE-TENSIONING OPERATION: OBSERVE
- C. FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING: OBSERVE
- D. FASTENERS ARE PRETENSIONED IN ACCORDANCE W/ THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES: OBSERVE

6. INSPECTION TASKS AFTER BOLTING:

- A. DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNS: PERFORM
- INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL: PERFORM

A. INSPECTION OF WELDING:

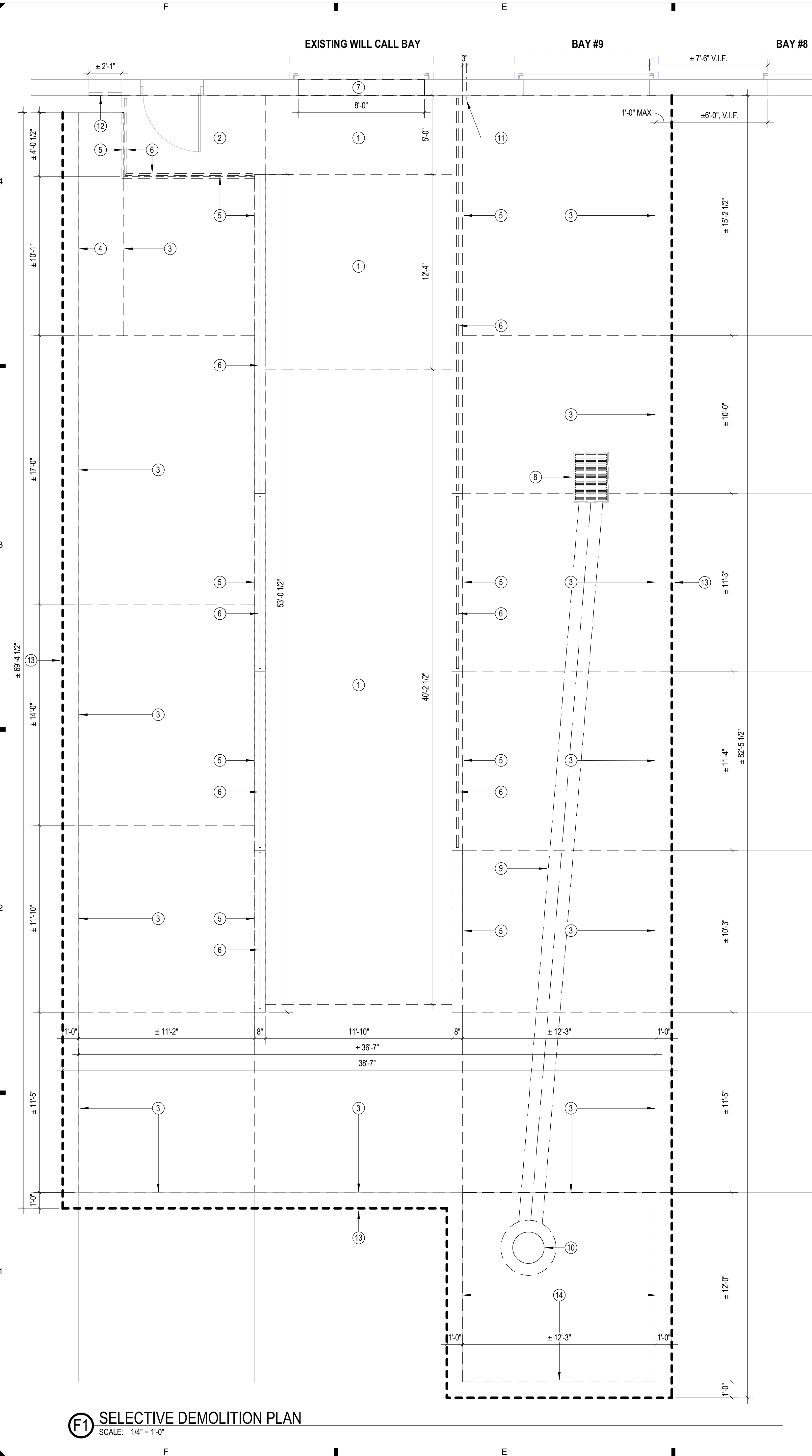
- 1) VERIFICATION OF WELDABILITY OF REINF. STEEL OTHER THAN ASTM A706: PERIODIC
- 2) REINFORCING STEEL RESISTING FLEXURE & AXIAL FORCES IN INTERMEDIATE & SPECIAL MOMENT FRAMES & BOUNDARY FOUNDATIONS OF SPECIAL STRUCTURAL WALLS OF CONCRETE & SHEAR REINFORCEMENT: CONT.
- 4) SHEAR REINFORCING STEEL: CONT.
- 5) OTHER REINFORCING STEEL: PERIODIC

CONCRETE

- 1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, & PLACEMENT: PERIODIC
- 2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE W/ IBC TABLE 1705.2.2 (ITEM 2b): PERIODIC
- 3. INSPECTION OF ANCHORS TO BE INSTALLED IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED: PERIODIC
- 4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: PERIODIC
- 5. VERIFYING USE OF REQUIRED DESIGN MIX: PERIODIC
- 6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP & AIR CONTENT TESTS, & DETERMINE THE TEMPERATURE OF THE CONCRETE: CONT.
- 7. INSPECTION OF CONCRETE & SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES: CONT.
- 8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMP. & TECHNIQUES: PERIODIC
- 9. INSPECTION OF PRESTRESSED CONCRETE: CONT.
- A. APPLICATION OF PRESTRESSING FORCES: CONT.
- B. ROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM: CONT.
- 10. ERECTION OF PRECAST CONCRETE MEMBERS: PERIODIC
- 11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST TENSIONED CONCRETE & PRIOR TO REMOVAL OF SHORES & FORMS FROM BEAMS & STRUCTURAL SLABS: PERIODIC
- 12. INSPECT FORMWORK FOR SHAPE, LOCATION & DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED: PERIODIC

PROGRAM FOOTNOTES:

- 1. THE ITEMS INDICATED ABOVE SHALL BE INSPECTED IN ACCORDANCE WITH CHAPTER 17 OF THE IBC BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE PROJECT SPECIFICATIONS & THE SPECIFIC STRUCTURAL NOTES SECTIONS. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING & INSPECTION REPORTS DIRECTLY TO THE ARCHITECT, STRUCTURAL ENGINEER, CONTRACTOR & BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET THE PROJECT SPECIFICATIONS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS.
- 2. CONT. - CONTINUOUS SPECIAL INSPECTION. SPECIAL INSPECTOR IS PRESENT WHEN & WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED. (IBC SECTION 1702.)
- 3. PERIODIC - PERIODIC SPECIAL INSPECTION. SPECIAL INSPECTOR IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING



**F1 SELECTIVE DEMOLITION PLAN**  
SCALE: 1/4" = 1'-0"

**GENERAL NOTES**

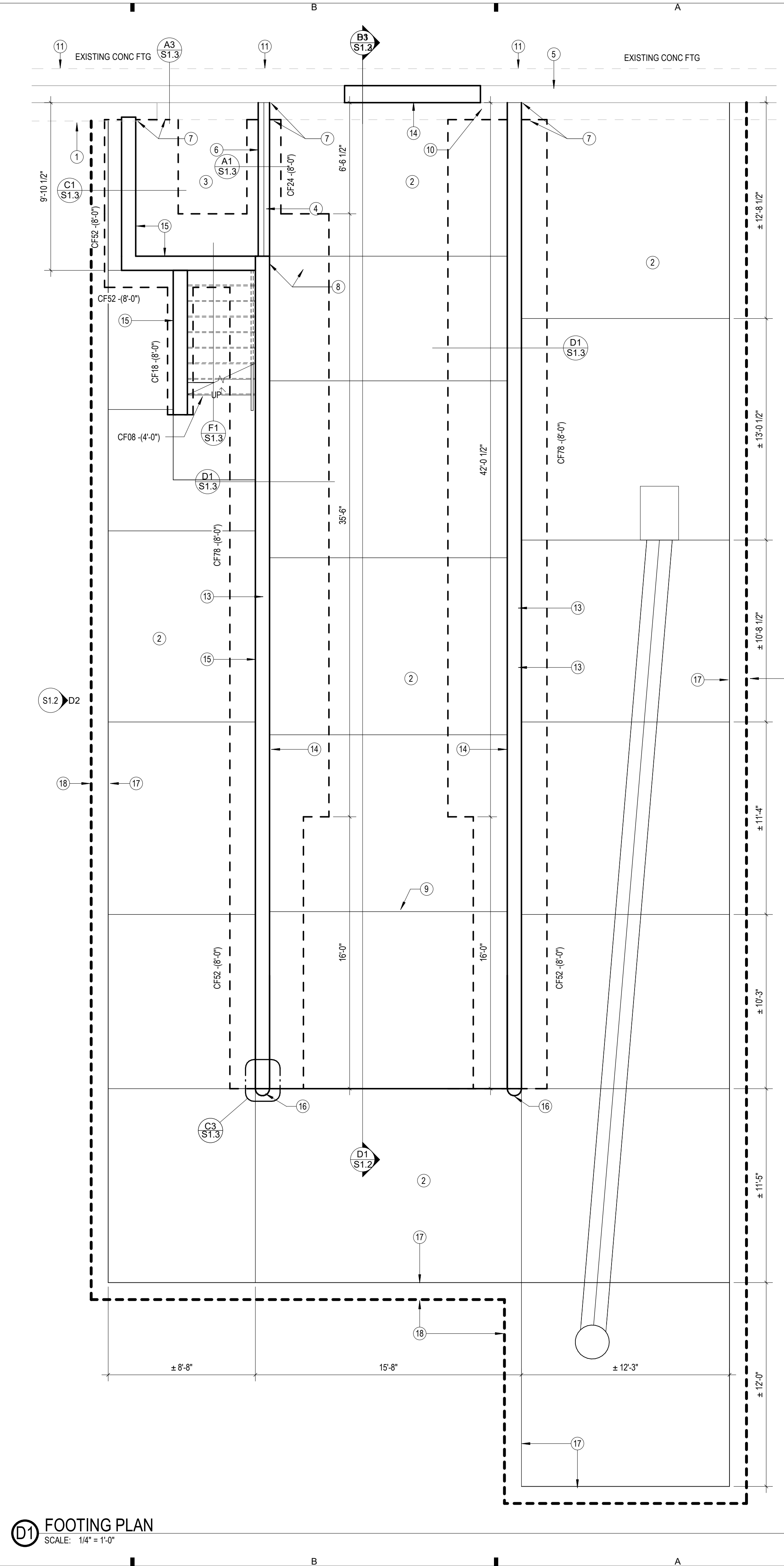
- A. FIELD VERIFY ALL EXISTING DIMENSIONS, SITE CONDITIONS, AND BUILDING CONDITIONS PRIOR TO WORK.
- B. THESE DEMOLITION DRAWINGS REPRESENT THE LAYOUT OF EXISTING CONDITIONS & MAJOR COMPONENTS. THEY ARE NOT TO BE CONSTRUED AS A COMPLETE REPRESENTATION OF MATERIALS & INCIDENTAL CONSTRUCTION.
- C. DEMOLISH AND/OR REMOVE ALL COMPONENTS AS NOTED ON THE DRAWINGS AND AS NECESSARY TO ACHIEVE NEW WORK IF NOT EXPLICITLY SHOWN AS DEMOLISHED. PATCH AND REPAIR ADJACENT SURFACES TO ACCOMMODATE NEW WORK. MAINTAIN ADJACENT SURFACES TO ORIGINAL CONDITIONS. ANY DAMAGE OF EXISTING CONDITIONS SHALL BE REPAIRED TO ORIGINAL OR BETTER CONDITION.
- D. ALL MATERIALS THAT HAVE BEEN DEMOLISHED SHALL BE REMOVED AND DISPOSED OF PROPERLY. NO DEMOLISHED MATERIALS SHALL BE STOCKPILED ON SITE.
- E. PROTECT OWNER'S PROPERTY AND PERSONS AT ALL TIMES. THIS INCLUDES ALL ITEMS AND SERVICES NECESSARY TO DEMOLISH OR DISMANTLE AND REMOVE ALL WALLS, EQUIPMENT, PIPING, AND APPURTENANCES WHICH WILL INTERFERE WITH NEW CONSTRUCTION. ALL ITEMS TO BE REMOVED SHALL BE COORDINATED WITH NEW CONSTRUCTION.
- F. ANY ITEMS NOT SHOWN TO BE DEMOLISHED THAT ARE DAMAGED DURING THE COURSE OF DEMOLITION OR CONSTRUCTION SHALL BE REPAIRED/REPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- G. COORDINATE ANY SYSTEMS SHUTDOWNS WHICH MAY BE REQUIRED WITH THE OWNER.
- H. PROVIDE APPROPRIATE TEMPORARY OR PERMANENT SUPPORT WHEREVER OPENINGS ARE CREATED IN EXISTING CONSTRUCTION AS REQUIRED.
- I. EXISTING CONSTRUCTION SHALL BE PROTECTED.
- J. BUILDING FINISH FLOOR ELEVATION = 0'-0"

**FOOTING PLAN NOTES**

1. EXISTING FOUNDATION WALL EXTENSION
2. 8" CONCRETE SLAB W/ #5 AT 1'-0" O.C. BOTH WAYS OVER 6" DRAINAGE FILL OVER COMPACTED SUBGRADE.
3. 6" CONCRETE SLAB WITH TOP AND BOTTOM MAT OF #5 @ 1'-0" O.C. EACH WAY OVER VOID FORMS OVER COMPACTED STRUCTURAL FILL. SPLICE ALL BARS TO BENT VERTICAL REINFORCEMENT IN PERIMETER WALLS. NORTH-SOUTH REINFORCEMENT TO BE ON THE BOTTOM OF REINFORCEMENT MATS.
4. PIN SLAB TO FROST SLAB WITH SMOOTH, GREASED 2'-0" LONG #6 DOWELS AT 2'-0" O.C. PROVIDE 5' MIN. DRILL DOVEL AND EPOXY WITH 6' MIN. EMBEDMENT.
5. EXISTING CONCRETE FOUNDATION WALL
6. 8" CONCRETE WALL - SEE SCHEDULE FOR REINFORCING
7. DRILL, DOVEL, AND EPOXY ALL PERPENDICULAR REINFORCEMENT BARS INTO EXISTING FOOTING, 6" EMBEDMENT. CLEAN AND SCARIFY FACE OF EXISTING FOOTING ACCORDING TO REQUIREMENTS IN GENERAL STRUCTURAL NOTES.
8. SEE DETAILS FOR REQUIRED SPLICING AT ALL FOUNDATION WALL AND FOOTING INTERSECTIONS, TYP.
9. SLAB CONTROL JOINT - SEE DETAILS.
10. 6" CONCRETE-FILLED STEEL PIPE BOLLARD. EXTEND DOWN TO FOOTING.
11. EXISTING FOOTING DEPTH AND THICKNESS BASED ON SITE INVESTIGATION PERFORMED BY CONTRACTOR. ALERT STRUCTURAL ENGINEER IF DIMENSIONS DISCOVERED DURING EXCAVATION DIFFER THAN THOSE SHOWN. DO NOT UNDERMINE EXISTING FOOTINGS, EITHER DIRECTLY OR WITHIN THE ANGLE OF INCIDENCE OF THE BEARING LOAD ON THE SOIL. CONTACT STRUCTURAL ENGINEER IF ANY UNDERMINING IS SUSPECTED.
13. GALVANIZED STEEL GUARDRAIL WITH HORIZONTALS AT 42" AND +21" FROM FINISHED SLAB ELEVATION.
14. PROVIDE EXPANSION JOINT WITH NO POSITIVE MECHANICAL ANCHORAGE BETWEEN SLAB AND WALL.
15. 12" CONCRETE WALL - SEE SCHEDULE FOR REINFORCING
16. CUT HSS12x0.375 WITH 1/2" EXPANSION ANCHOR AT 2'-0" O.C., FULLY GROUTED AND FULL HEIGHT OF CONCRETE WALL
17. SEE DETAILS FOR REQUIRED SMOOTH BAR DOWELS AT JOINT BETWEEN NEW AND EXISTING CONCRETE SLABS.
18. CHAIN LINK CONSTRUCTION FENCE - SEE SELECTIVE DEMOLITION PLAN

**DEMO PLAN NOTES**

1. REMOVE RAMP CONCRETE SLAB
2. REMOVE CONCRETE SLAB
3. REMOVE CONCRETE PANEL TO INDICATED JOINT LINE
4. SAWCUT AND REMOVE CONCRETE PANEL, EXTEND SAW CUT LINE TO FOUNDATION
5. REMOVE RETAINING WALL AND ASSOCIATED FOOTING
6. REMOVE GUARDRAILS
7. REMOVE EXISTING THRESHOLD CURB DOWN TO TOP OF FOUNDATION WALL; PREP FOR NEW THRESHOLD CURB
8. REMOVE EXISTING DRAIN INLET AND SALVAGE FOR REINSTALLATION
9. EXISTING 18" DRAIN LINE, APPROXIMATELY 8'-8" BELOW FINISH FLOOR. PROTECT FOR DURATION OF CONSTRUCTION, ENSURE SLOPE IS MAINTAINED TO PROVIDE POSITIVE DRAINAGE TO COLLECTOR LINE TO SOUTH
10. EXISTING STORM SEWER MANHOLE - PROTECT DURING CONSTRUCTION
11. EXISTING HOSE BIBB IN EXTERIOR WALL, ±8" AFF - PROTECT DURING CONSTRUCTION
12. GRIND FLUTED FINISH OF PRECAST FLUSH TO CREATE SMOOTH SURFACE FOR NEW STOOP SLAB
13. MIN. 3'-FT HIGH CHAIN LINK CONSTRUCTION FENCE LOCATED WITHIN 1'-0" OF CUT EDGE OF PAVING
14. REMOVE EXISTING CRACKED CONCRETE PANEL TO INDICATED JOINT LINES; PROTECT AND MAINTAIN LOCATION OF MANHOLE DURING CONSTRUCTION



**D1 FOOTING PLAN**  
SCALE: 1/4" = 1'-0"

PROJECT	ISSUE DATE	06/22/2026
	PROJECT NUMBER	9534.00
	REVISIONS:	02/2026

REV.	DATE	DESCRIPTION

BIDDING DOCUMENTS

DEMO & FOOTING PLAN



