IMCC DOOR REPLACEMENT IOWA DEPARTMENT OF ADMINISTRATIVE SERVICES

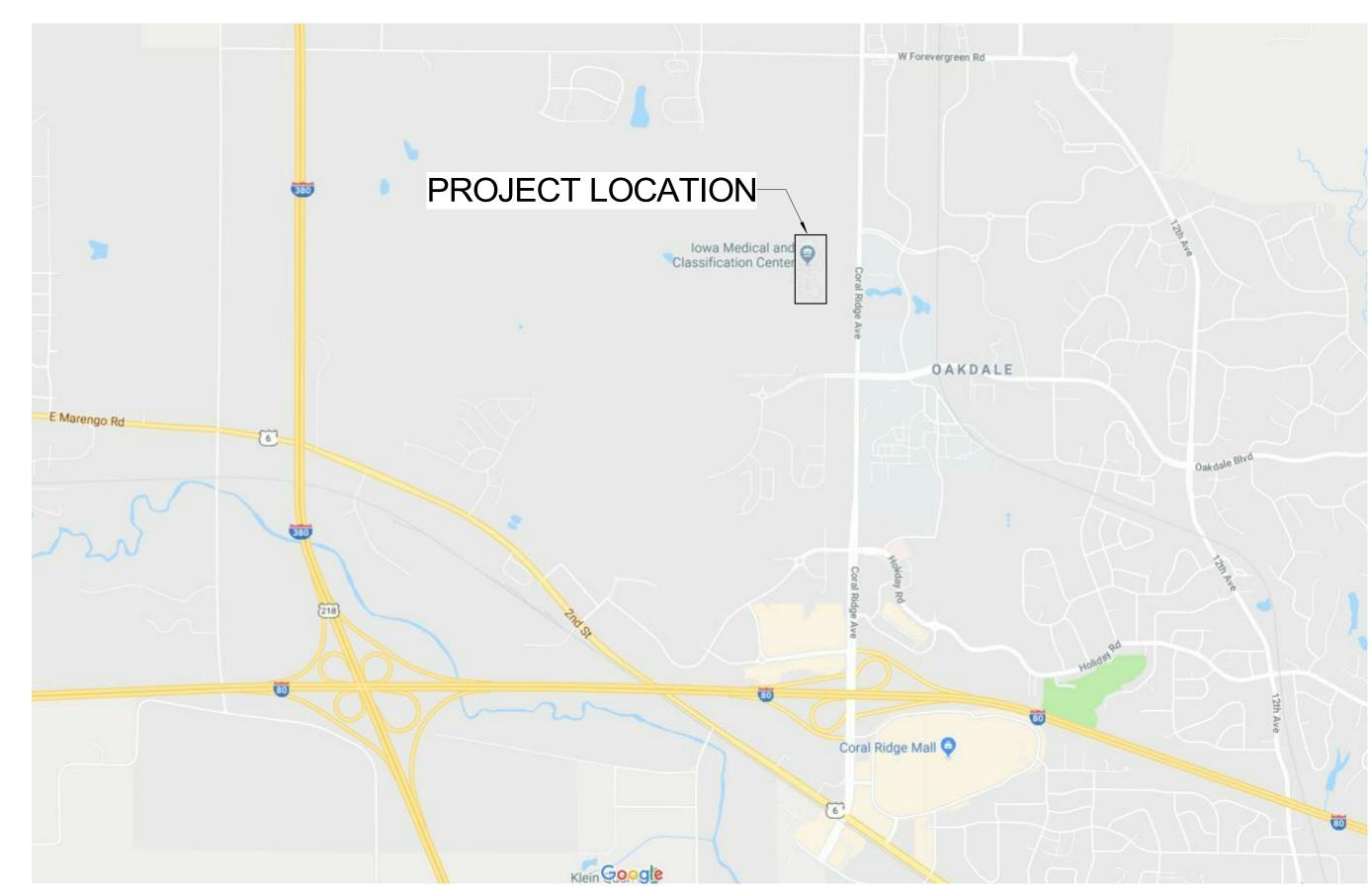
2700 Coral Ridge Ave Coralville, IA 52241

SHEET INDEX

GENERAL

CERTIFICATIONS

ARCHITECT					
	I HEREBY CERTIFY THAT THE PORTION SUBMISSION DESCRIBED BELOW WAS MY DIRECT SUPERVISION AND RESPON LICENSED ARCHITECT UNDER THE LAV	PREPARED BY ME OR UNDER ISIBLE CHARGE. I AM A DULY			
		3/22/2019			
	SIGNATURE	DATE			
	PRINTED OR TYPED NAME Richard C. Cleaveland				
	LICENSE NUMBER 06537				
	MY LICENSE RENEWAL DATE IS JUNE 3	_{0,} 2020			
	PAGES, SHEETS OR DIVISIONS COVERE	ED BY THIS SEAL:			





IOWA DEPARTMENT OF 109 SE 13TH STREET DES MOINES, IA 50319

ADMINISTRATIVE SERVICES

C DOOR REPLACEME

VED: RCC

FOR: CONSTRUCTION

03/13/2019

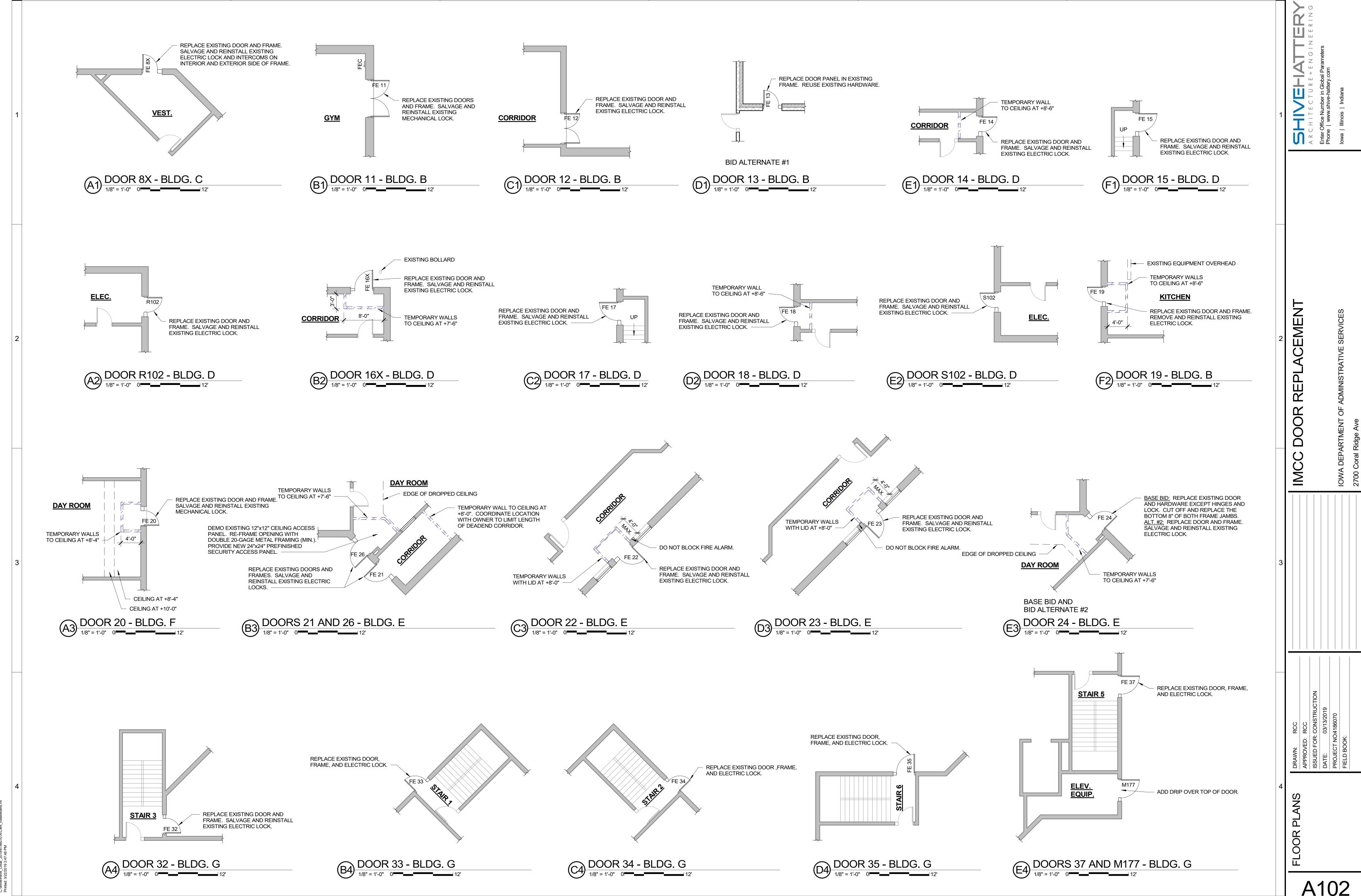
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FACILITY MAP

A101

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C. AMERICAN INSTITUTE OF STEEL CONSTRUCTION - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360)

ALLOWABLE STRENGTH DESIGN (ASD) D. AMERICAN SOCIETY OF CIVIL ENGINEERS AND STRUCTURAL ENGINEERING INSTITUTE (ASCE/SEI 7) - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

E. AMERICAN WELDING SOCIETY D1.1

MINIMUM FROST PROTECTION DEPTH MEASURED FROM GRADE EXTERIOR FOOTING ADJACENT TO HEATED AREA (-3'-6")EXTERIOR FOOTING AT UNHEATED AREA (-4'-0")

GENERAL NOTES

I. THE GENERAL STRUCTURAL NOTES ARE INTENDED TO SUPPLEMENT THE DRAWINGS AND SPECIFICATIONS. SHOULD CONFLICTS EXIST BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS NOTIFY THE ENGINEER OF ANY SUCH CONFLICTS. 2. THE INTENT OF THESE PLANS AND NOTES IS TO PRESENT THE PROJECT

REQUIREMENTS. MAJOR DETAILS HAVE BEEN SHOWN ON THE DRAWINGS. HOWEVER, CERTAIN MINOR DETAILS MUST BE WORKED OUT IN THE FIELD OR SHOP DRAWING PROCESS BY THE CONTRACTOR.

3. UNLESS NOTED OTHERWISE, DETAILS SHOWN ON DRAWINGS ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.

4. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW THE APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

EXISTING CONDITIONS A. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS RELATING TO EXISTING CONSTRUCTION AND EXISTING SERVICES ON SITE. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO PROCEEDING WITH THE WORK, REPORT ANY DISCREPANCIES WITH THE CONTRACT DOCUMENTS TO THE ARCHITECT BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

B. DURING CONSTRUCTION THE CONTRACTOR MAY ENCOUNTER EXISTING CONDITIONS WHICH ARE NOT KNOWN OR ARE AT VARIANCE WITH PROJECT DOCUMENTATION (DISCOVERY). SUCH CONDITIONS MAY INTERFERE WITH THE NEW CONSTRUCTION OR REQUIRE PROTECTION AND/OR SUPPORT OF EXISTING WORK DURING CONSTRUCTION. IT MAY ALSO CONSIST OF DAMAGED OR DETERIORATION OF STRUCTURAL MATERIALS OR COMPONENTS WHICH COULD JEOPARDIZE THE STRUCTURAL INTEGRITY OF THE BUILDING(S). THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ALL DISCOVERIES WHICH MAY INTERFRERE WITH THE PROPER EXECUTION OF THE WORK OR JEOPARDIZE THE STRUCTURAL INTEGRITY OF THE BUILDING(S) PRIOR TO PROCEEDING WITH THE WORK RELATED TO SUCH DISCOVERIES.

D. DURING THE CONSTRUCTION PROCESS, IT SHALL BE SOLELY THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN THE INEGRITY OF THE EXISTING STRUCTURE AND TO PROTECT IT FROM DAMAGE.

CONTRACTOR SHALL INVESTIGATE THE SITE DURING EARTHWORK OPERATIONS FOR FILL MATERIAL OR BURIED STRUCTURES. IMMEDIATELY, NOTIFY THE ARCHITECT IF ANY SUCH MATERIALS OR STRUCTURES ARE

CONTRACTOR SHALL COORDINATE WORK SCHEDULES WITH THE OWNER TO ESTABLISH CONSTRUCTION SEQUENCING AROUND ANY OCCUPIED AREAS. CONTRACTOR SHALL NOT PROCEED TO OCCUPIED AREAS UNTIL AUTHORIZED BY THE OWNER.

7. ALL ELEMENTS AND SURFACES DAMAGED BY DEMOLITION, BUT NOT SCHEDULED FOR REMOVAL SHALL BE REPAIRED AND REFINISHED TO MATCH THE ADJACENT SURFACES AT NO ADDITIONAL COST TO THE OWNER.

CONTRACTOR SHALL REMOVE ALL DEBRIS AND WASTE MATERIALS RESULTING FROM CONSTRUCTION FROM THE SITE, UNLESS NOTED OTHERWISE.

CONTRACTOR SHALL MINIMIZE CREATION OF DUST, DIRT AND WINDBORNE DEBRIS FROM BLOWING ACROSS THE SITE AND ONTO ADJACENT SITES.

CAST-IN-PLACE CONCRETE

1. ALL CONCRETE SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTES PUBLICATIONS: ACI 301, ACI 305.1, ACI 306.1, ACI 315, AND ACI 318 UNLESS NOTED OTHERWISE.

2. CONCRETE COMPRESSIVE STRENGTH

(28 DAY)(F'c)

FOOTINGS 4000 PSI FOUNDATION WALLS AND PIERS 4000 PSI SLAB ON GRADE 4000 PSI 3. CONCRETE REINFORCEMENT STANDARDS:

DEFORMED BARS ASTM A615 Fy = 60 KSI

4. ALL CONCRETE SHALL BE STONE AGGREGATE UNLESS NOTED OTHERWISE. SUBMIT MIX DESIGN AND DOCUMENTATION FOR APPROVAL PER ACI 318.

REINFORCEMENT PROTECTION

A. CONCRETE PLACED AGAINST EARTH B. CONCRETE PLACED IN FORMS BUT EXPOSED TO WEATHER OR EARTH:

a. BARS #5 AND SMALLER 1 1/2" b. BARS LARGER THAN #5

WHERE REQUIRED, DOWELS SHALL MATCH THE SIZE, NUMBER AND SPACING OF THE MAIN REINFORCING UNLESS NOTED OTHERWISE ALL SPLICES, STANDARD HOOKS, AND DEVELOPMENT LENGTHS TO BE PER THE

CORNERS. ALL SPLICES SHALL BE BY CONTACT LAP. 8. ALL SPLICES SHALL BE A CLASS "B" TENSION SPLICE AS DEFINED IN ACI 318. PROVIDE LAP SPLICES LENGTHS AS FOLLOWS:

REFERENCED EDITION OF ACI 318. MAKE BARS CONTINUOUS AROUND

	4000 PSI				
BAR SIZE	TYPICAL	TOP BARS			
#3	19"	25"			
#4	25"	33"			
#5	31"	41"			
#6	37"	49"			
#7	54"	71"			
#8	62"	81"			
#9	70"	91"			
#10	79"	102"			

#11 87" 114"

LAP SPLICE LENGTHS GIVEN, ASSUME CLEAR SPACING BETWEEN BARS OF 2 BAR DIAMETERS, AND A MINUMUM CLEAR COVER OF 1 BAR DIAMETER. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12" ON FRESH CONCRETE BENEATH THE BARS.

9. WALLS AND GRADE BEAMS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE,

UNLESS APPROVED BY THE ARCHITECT

12. REINFORCING STEEL SHALL BE SECURELY FASTENED INTO FORMS PRIOR TO POURING CONCRETE. WET SETTING OF REINFORCING STEEL WILL NOT BE ACCEPTED PER ACI.

13. CONCRETE MIX - FOOTINGS AND PAVEMENT

COARSE AGGREGATE 100% PASSING 1" SIEVE FINE AGGREGATE 100% PASSING 3/8" SIEVE WATER/CEMENT RATIO 0.45 SLUMP 4" +/- 1" AIR CONTENT 6% +/- 1.5%

SUBMITTALS

1. CONRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL THE FOLLOWING SUBMITTALS FOR EACH MATERIAL INDICATED BELOW.

CONCRETE REINFORCING

A. SUBMIT CONCRETE REINFORCEMENT SHOP DRAWINGS IN ACCORDANCE WITH ACI 315 FOR APPROVAL.

 a. DETAIL BARS IN ACCORDANCE WITH "ACI DETAILING MANUAL" PUBLICATION SP-66 AND THE LATEST EDITION OF ACI 318 "BUILDING

CODE REQUIREMENTS FOR REINFORCED CONCRETE". 3. CAST-IN-PLACE CONCRETE

A. SUBMIT CONCRETE MIX DESIGNS FOR EACH APPLICATION LOCATION INDICATED IN THE DRAWINGS. B. SUBMIT SHOP DRAWING OF EMBEDDED STEEL ELEMENTS AND

FOUNDATIONS

1. CONTRACTOR SHALL ACCOUNT FOR PUMPING OF WATER FROM THE

EXCAVATION DUE TO SURFACE WATER, GROUND WATER AND SEEPAGE. 2. ALL FOOTINGS SHALL BE PLACED ONTO FIRM UNDISTURBED SOIL OR ACCEPTABLE COMPACTED BACKFILL AS OUTLINED IN THE PROJECT

SPECIFICATIONS. 3. FOOTING ELEVATIONS SHOWN DESIGNATE THE MINIMUM DEPTH OF THE FOOTING WHERE THE ALLOWABLE SOIL BEARING IS EXPECTED. LOCALIZED AREAS OF UNACCEPTABLE SOILS OR POOR COMPACTION MAY BE DISCOVERED DURING THE EXCAVATION PROCESS REQUIRING OVEREXCAVATION AND BACKFILL WITH ACCEPTABLE FILL. FOOTING EXCAVATIONS SHALL BE LOWERED TO REACH SOIL MEETS THE DESIGN BEARING PRESSSURE AND APPROVED BY THE GEOTECHNICAL SPECIAL INSPECTION AGENCY.

4. ACCEPTABLE BACKFILL MATERIAL SHALL BE PLACED IN LIFTS NOT TO EXCEED

EIGHT (8) INCHES IN LOOSE THICKNESS. 5. FOR FOOTING AND FOUNDATIONS, THE SUBGRADE OR FILL MATERIAL SHALL BE COMPACTED AND VERIFIED TO MEET 98% STANDARD PROCTOR MAXIMUM DRY DENSITY ACCORDANCE WITH ASTM D698. FOR RELATIVELY COHESIONLESS GRANULAR FILL WHICH HAS A PERCENT PASSING THE #200 SIEVE LESS THAN 10 PERCENT AND HAS ONLY A SLIGHT SENSITIVITY TO MOISTURE CHANGES, COMPACTION SHALL BE 75 PERCENT RELATIVE DENSITY IN ACCORDANCE WITH ASTM D4253 AND D4254. IF COMPACTION DOES NOT COMPLY, CONTRACTOR SHALL RECOMPACT AREA AND UNITL TEST RESULTS ARE PASSING. AN AREA EXHIBITING WEAKNESS SUCH AS RUTTING OR PUMPING SHALL BE REMOVED AND REPLACED WITH COMPACTED GRANULAR FILL

6. FOOTING EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHINCAL SPECIAL INSPECTION AGENCY BEFORE CONCRETE IS PLACED. CONTRACTOR SHALL NOTIFY INSPECTION AGENCY WHEN EXCAVATION IS READY FOR TESTING. INSPECTION AGENCY SHALL PROVIDE A WRITTEN REPORT OF TEST RESULTS AND COMPLIANCE TO THE OWNER.

7. ACCEPTABLE SOIL SHALL BE DEFINED AS MEETING ASTM D2487 SOIL CLASSIFICATION GROUPS GW, GP, GM, SW, SP, SM OR A COMBINATION OF

UNACCEPTABLE SOILS SHALL BE DEFINED AS MEETING ASTM D2487 SOIL CLASSIFICATION GROUPS GC, SC, ML, MH, CL, CH, OL, OH, PT OR A COMBINATION OF THESE TYPES. GROUPS CL AND ML MAY BE ACCEPTABLE IF THE LIQIUD LIMIT IS LESS THAN 45 AND THE PLASTICITY INDEX IS LESS THAN 20.

9. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT

ANY FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADES BEFORE AND AFTER PLACING THE CONCRETE AND UNTIL SUCH SUBGRADE IS FULLY PROTECTED BY THE PERMANENT BUILDING ENCLOSURE AND THE SPACE IS CONDITIONED TO REMAIN ABOVE FREEZING.

10. CONCRETE FOOTINGS AND SLABS SHALL NOT BE PLACED ON OR AGAINST SUBGRADES CONTAINING FROST, SNOW OR ICE. FROZEN SUBGRADES SHALL BE COMPLETELY THAWED AND RECONDITIONED BEFORE CONCRETE MAY BE

11. REPEATED HEAVY CONSTRUCTION TRAFFIC OVER EXPOSED SUBGRADE WILL CAUSE RUTTING AND PUMPING WHEN SOIL IS ABOVE THE OPTIMUM MOISTURE CONTENT. AVOID EXCESS CONSTRUCTION ACTIVITY ON WET SOILS. IF SUBGRADE IS ABOVE THE OPTIMUM MOISTURE CONTENT DURING CONSTRUCTION, THEN DRYING OF THE SOIL SHALL BE CONDUCTED BY DISKING, SCARAFICATION, AND AERATION.

12. SOILS WITH A MOISTURE CONTENT ABOVE THE OPTIMUM LEVEL SHALL BE REMOVED AND REPLACED WITH COMPACTED GRANULAR FILL.

13. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER IMMEDIATELY IF THE EXISTING FOUNDATIONS VARY FROM THAT SHOWN ON THE DRAWINGS.

CONCRETE PAVING

1. COMPACTION SHALL BE TESTED AND VERIFIED TO MEET 98% STANDARD PROCTOR MAXIMUM DRY DENSITY ACCORDANCE WITH ASTM D698. FOR RELATIVELY COHESIONLESS GRANULAR FILL WHICH HAS A PERCENT PASSING THE #200 SIEVE LESS THAN 10 PPERCENT AND HAS ONLY A SLIGHT SENSITIVITY TO MOISTURE CHANGES, COMPACTION SHALL BE 75 PERCENT RELATIVE DENSITY IN ACCORDANCE WITH ASTM D4253 AND D4254. IF COMPACTION DOES NOT COMPLY, CONTRACTOR SHALL RECOMPACT AREA AND UNITL TEST RESULTS ARE PASSING. AN AREA EXHIBITING WEAKNESS SUCH AS RUTTING OR PUMPING SHALL BE REMOVED AND REPLACED WITH COMPACTED GRANULAR FILL.

2. CONCRETE SLAB ON GRADE SHALL BE PLACED ON A 6" WELL GRADED COMPACTED GRANULAR FILL SUB-BASE.

3. PLACE ALL SLABS ON GRADE WITH AN APPROVED JOINT PATTERN SUBMITTED BY CONTRACTOR AND APPROVED BY ENGINEER OR AS SHOWN ON DRAWINGS. SEQUENCE OF CONSTRUCTION AND CONTROL JOINTS SHALL BE PLACED TO MINIMIZE SHRINKAGE CRACKS.

4. CONCRETE SLAB ON GRADES SHALL HAVE CONTROL JOINTS SAW CUT OR TOOLED . LOCATE JOINT ALONG COLUMN CENTER LINES WITH INTERMEDIATE JOINTS AT A MAXIMUM SPACING OF 36 TIMES THE SLAB THICKNESS, UNLESS NOTED OTHERWISE. SLAB JOINT PANELS SHALL HAVE A MAXIMUM LENGTH TO WIDTH RATIO OF 1.5:1. DO NOT STAGGER OR OFFSET JOINTS. PROVIDE ADDITIONAL JOINTS AT RE-ENTRANT CORNER. IF RE-ENTRANT CORNERS ARE UNAVOIDABLE, THEN ADDITIONAL REINFORCING COMPRISED OF (2) #4 BARS x 3'-0" SHALL BE PLACED IN THE CENTER OF THE SLAB DIAGONAL TO THE RE-ENTRANT CORNER CONDITION. PROVIDE SHOP DRAWING OF CONTROL JOINT PATTERN AND CORNER REINFORCING.

5. SAWCUT JOINTS AS SOON AS SURFACE WILL ALLOW WITHOUT EDGES RAVELING BUT PRIOR TO THE NEXT DAY AFTER THE POUR.

6. PAVEMENT FINISHES SHALL BE BROOM FINISHED UNLESS NOTED OTHERWISE.

WATER REPELLENT

1. PROVIDE NON-GLOSSY, COLORLESS, PENETRATING, NON-YELLOWING SILANE-SILOXANE BLEND WATER REPELLENT SEALER.

2. ACCEPTABLE PRODUCTS INCLUDE: ADVANCED CHEMICAL TECHNOLOGIES, INC; SIL-ACT DRI-TRETE WB PROSOCO, INC.; CONSOLIDECK SALT GUARD TNEMEC, INC.; CHEMPROBE PRIM-A-PEL 20, SERIES 636

OTHER PRODUCTS APPROVED IN WRITING BY ARCHITECT. 3. PREPARE AND APPLY SEALER TO CONCRETE SURFACE IN ACCORDANCE WITH MANUFACTURERS WRITTEN INSTRUCTIONS AFTER CONCRETE HAS CURED FOR A MINIMUM OF 60 DAYS.

STATEMENT OF SPECIAL INSPECTIONS

This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to the structural components of this project. If applicable, it includes Requirements for Seismic Resistance and/or Requirements for Wind Resistance. This Statement of Special Inspections Encompasses the following disciplines:

D

[] Mechanical / Electrical / Plumbing [X] Structural] Architectural [] Other

The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge. A Final Report of Special Inspections documenting completion of all required Special Inspections, testing and corrections of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy. Job site safety and means and methods of construction are solely the responsibility of the Contractor.

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designations shall appear below the Agent on the Schedule.

Structural Engineer - A licensed SE or PE specializing in the design of building structure

PE/GE Geotechnical Engineer - A licensed PE specializing in soil mechanics and foundations Engineer-In-Training - A graduate engineer who has passed the Fundaments of Engineering examination

American Concrete Institute (ACI) Certification ACI-CFTT Concrete Field Testing Technician - Grade 1

ACI-CCI Concrete Construction Inspector ACI-LTT Laboratory Testing Technician - Grade 1 & 2 ACI-STT Strength Testing Technician

American Welding Society (AWS) Certification AWS-CWI Certified Welding Inspector

AWS/AISC-SSI Certified Structural Steel Inspector International Code Council (ICC) Certification

ICC-RCSI Reinforced Concrete Special Inspector

National Institute of Certification in Engineering Technologies (NICET) NICET-CT Concrete Technician - Levels I, II, III, & IV NICET-ST Soils Technician - Levels I, II, III & IV

NICET-GET Geotechnical Engineering Technician - Level I, II, III & IV

5. Review concrete batch tickets and verify

that allowed by the mix design

(ASTM C1064).

that water added at the site does not exceed

6. Test concrete compressive strength (ASTM

(ASTM C231 or C173) and temperature

. Inspect placement of concrete. Verify that

segregation or contamination. Verify that

concrete is properly consolidated. 8. Inspection for maintenance of specified

curing temperature and techniques

location and dimensions

9. Inspection of formwork for shape, lines,

compliance with construction documents

compliance with approved mix design. Verify Field Inspection

C31 & C39), slump (ASTM C143), air-content Field Inspection

concrete conveyance and depositing avoids Field Inspection

10. Concrete strength testing and verification of Field testing and review of

CONTRACTOR'S RESPONSIBILITY REGARDING INSPECTIONS

1. The Contractor is responsible for scheduling a pre-construction meeting (scheduled at least 5 business days before start of construction). Meeting should include all responsible parties (A/E, SI's, Field Inspector). Meeting is for entire project,

2. Pre-construction meeting is to be conducted by the contractor with meeting minutes to be taken and distributed to all members attending. Meeting minutes to include a sign-in sheet for all parties

3. The contractor is responsible for scheduling inspections and tests. Sufficient notice and lead time must be allowed for the inspection and testing to be performed without impending construction operations.

4. The contractor must cooperate with the inspections and testing agencies. Safe access must be provided to all inspection and test to be performed. This may require the contractor to provide scaffolding, ladders or lifts. 5. When deficiencies are identified, the contractor must take corrective actions to comply with the contract documents or

remedy the deficiencies as directed by the registered design professional. 6. The special inspection and quality assurance program does not relieve the contractor of his or her responsibility to

7. The contractor is responsible for testing services that are required for material submittals and that not part of the special inspections program (e.g. aggregate tests, concrete mix designs, testing of controlled fill, materials, etc.).

ONCRETE CONSTRUCTION S	SERVICE	EXTENT	AGENT
1. Inspect size, spacing, cover, positioning and grade of reinforcing steel. Verify that reinforcing bars are free of form oil or other deleterious materials. Inspect bar laps and mechanical splices. Verify that bars are adequately tied and supported of chairs or bolsters.	Field Inspection	Periodic	ACI-CCI ICC-RCSI
2. Reinforcing steel welding	Field Inspection		
Verification of weldability of steel other than ASTM A707		Periodic	
 Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement 		Continuous	
c. Shear reinforcement		Continuous	
Inspect size, positioning and embedment of anchor rods. Inspect concrete placement and consolidation around anchors	ield Inspection	Periodic	
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field Inspection	Periodic or as required by the research report issued by an approved source	ACI-CCI ICC-RCSI

Field Inspection

Field Inspection

ACI-CCI

ICC-RCSI

ACI-CFTT

ACI-STT

ACI-CCI

ICC-RCSI

ACI-CCI

ICC-RCSI

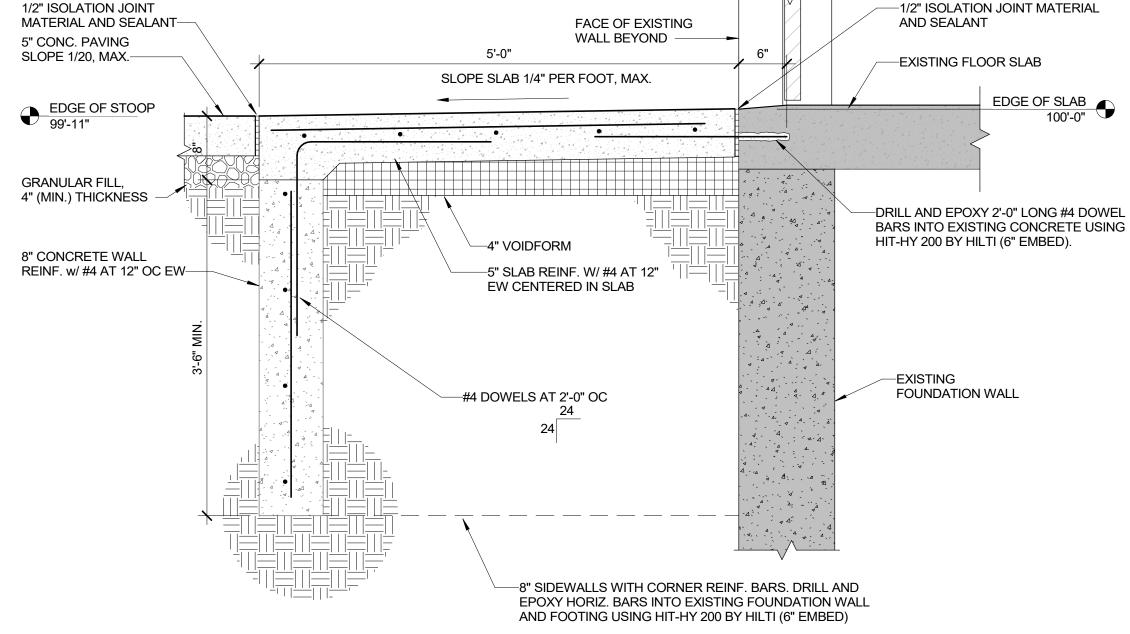
ACI-CCI

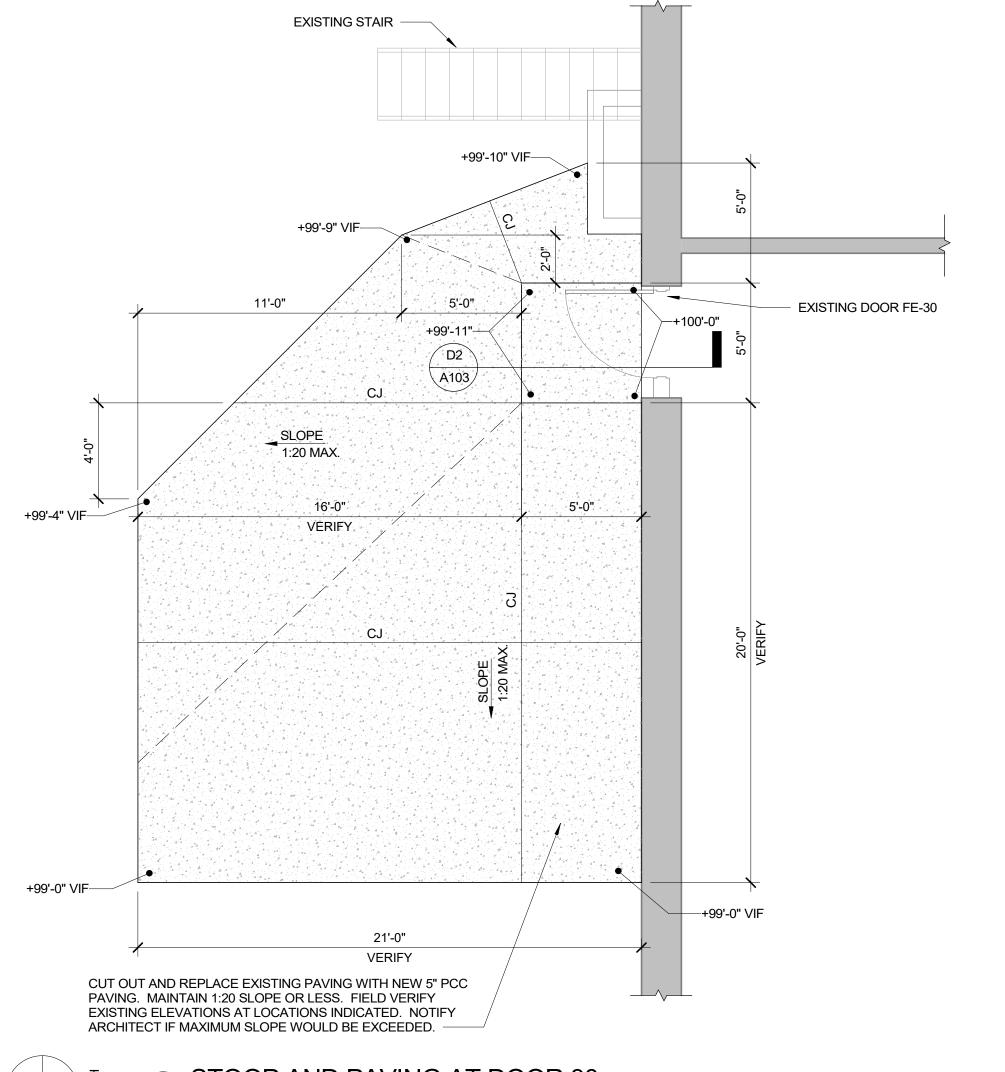
ICC-RCSI

Periodic

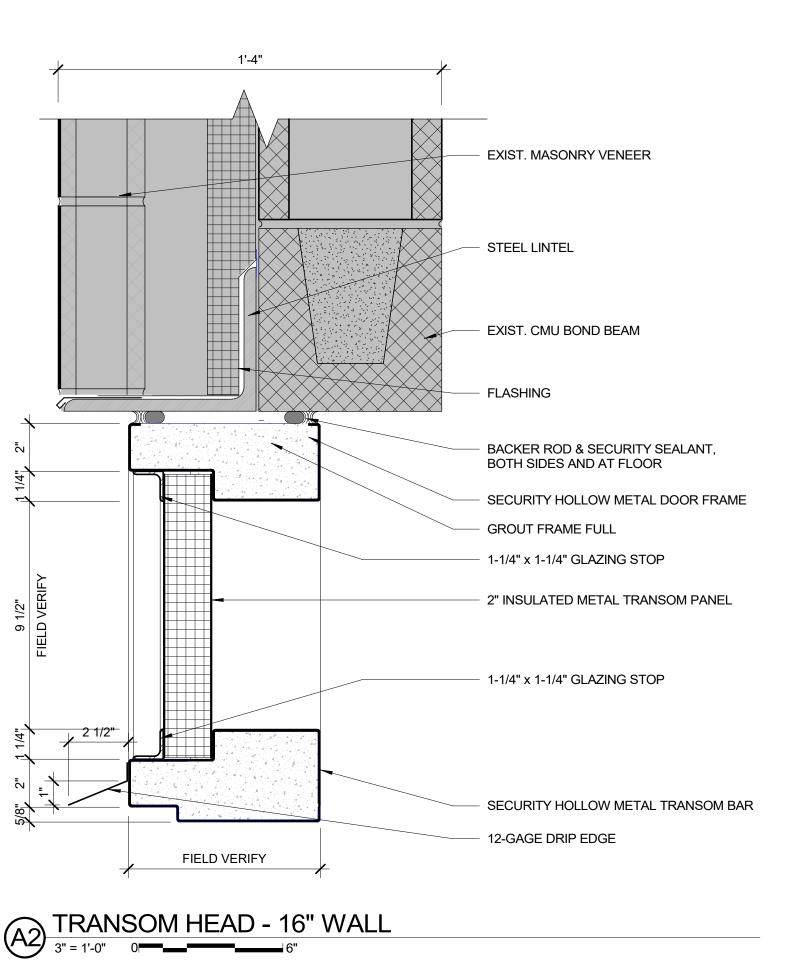
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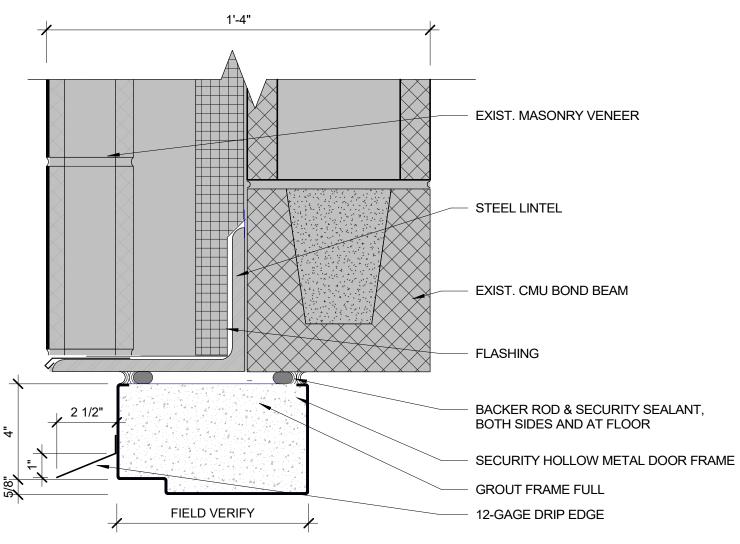
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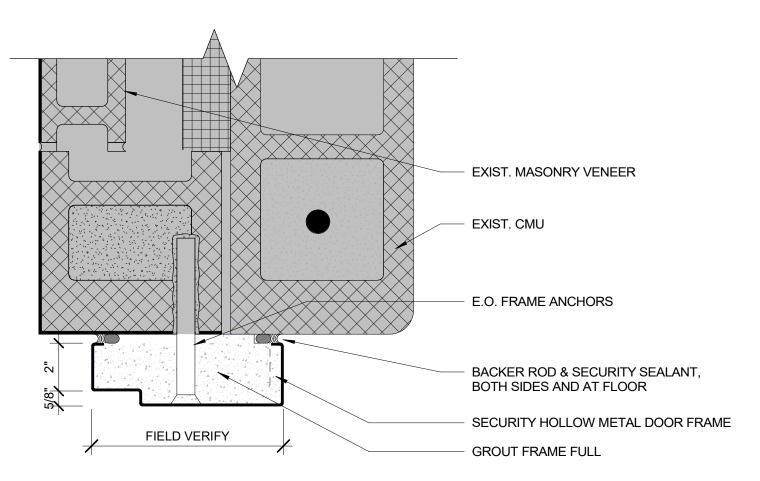


STOOP AT EXISTING FOUNDATION WALL

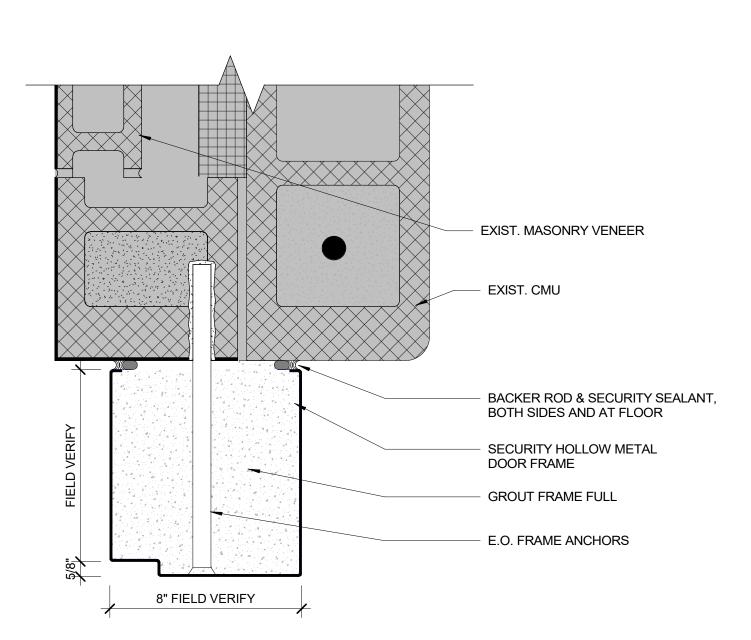












SECURITY DOOR STRIKE JAMB - 16" WALL

3" = 1'-0" 0 6"

FINISHED OPENING SIZE		LEAF			FRAME			DETAILS						
												STRIKE	-	
MARK	WIDTH	HEIGHT	EIGHT THICK	CONFIGURATION	TYPE	MTRL	GLAZ	MTRL-TYPE	GLAZ	HEAD	HINGE JAMB	JAMB	HDWR	REMARKS
FE 8X	3' - 0"	7' - 0"	1 3/4"		NV	SHM	GL1	SHM-004 : F11		A2/A601	A3/A601	A4/A601	2	3
FE 11	7' - 4"	7' - 0"	1 3/4"	PAIR 3' - 8" /	F/F	SHM		SHM-004 : F2		E2/A601	E3/A601	E4A601	5	9
FE 12	6' - 0"	7' - 0"	1 3/4"	PAIR 3' - 0" /	F/F	SHM		SHM-004 : F2		A2/A601	A3/A601	A4/A601	6	3
FE 13	3' - 0"	7' - 0"	1 3/4"		F	SHM		Existing : F8						BID ALT. #1
FE 14	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	8
FE 15	3' - 8"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	8
E 16X	3' - 8"	7' - 0"	1 3/4"		NV	SHM	GL1	SHM-004 : F10		B2/A600	B3/A600	B4/A600	2	4
FE 17	3' - 8"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	8
E 18	3' - 0"	7' - 0"	1 3/4"		NV	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	8
E 19	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	5
E 20	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-0018 : F10		C2/A601	C3/A601	C4A601	3	9
E 21	3' - 6"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F12		B2/A600	B3/A600	B4/A600	2	3
E 22	3' - 6"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F12		B2/A600	B3/A600	B4/A600	2	4
FE 23	3' - 6"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F12		B2/A600	B3/A600	B4/A600	2	4
FE 24	3' - 6"	7' - 0"	1 3/4"		NV	SHM	GL1	SHM-004 : F12		B2/A600	B3/A600	B4/A600	4	2, 3, BASE BID AND BID ALT. #2
E 26	3' - 6"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F12		B2/A600	B3/A600	B4/A600	2	3
FE 32	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F8		B2/A601	B3/A601	B4/A601	2	3
FE 33	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F8		B2/A601	B3/A601	B4/A601	1	3
E 34	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F8		B2/A601	B3/A601	B4/A601	1	6
E 35	3' - 8"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F8		B2/A601	B3/A601	B4/A601	1	3
E 37	3' - 8"	7' - 0"	1 3/4"		F	SHM		SHM-004 : F8		B2/A601	B3/A601	B4/A601	1	7
M177	3' - 8"	7' - 0"	1 3/4"		F	SHM		Existing : F8					7	1
R102	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	6
S102	3' - 0"	7' - 0"	1 3/4"		F	SHM		SHM-102 : F11	MP1	A2/A600	B3/A600	B4/A600	2	3

AS SCHED. AS SCHED. 5"8" SG-1 TYPE F TYPE NV

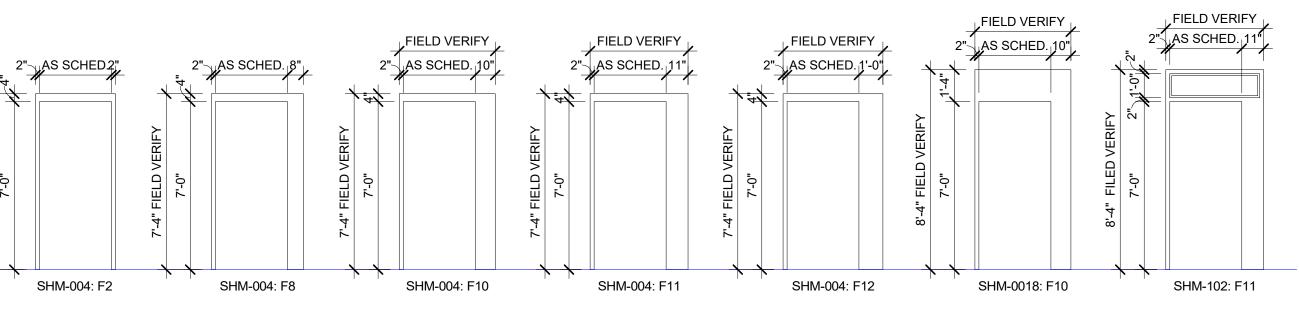
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NOTE: ALL GLAZING DIMENSIONS TO CLEAR LITE OPENING





- 1. EXISTING DOOR AND FRAME. SEE HARDWARE SET FOR NEW HARDWARE.
- 2. REPLACE BOTTOM 8" OF FRAME.
- 3. WIRING HAS AN IN-LINE CONNECTOR AT JUNCTION BOX WITHIN 10' OF DOOR.
- 4. WIRING HAS AN IN-LINE CONNECTOR AT JUNCTION BOX WITHIN 20' OF DOOR.
- WIRING HAS AN IN-LINE CONNECTOR AT JUNCTION BOX APPROXIMATELY 150' FROM DOOR.
- 6. JUNCTION BOX WITHIN 10' OF DOOR. WIRING IS PULLED TIGHT THROUGH JUNCTION BOX. IMCC ELECTRICIAN WILL ASSIST WITH TEMPORARILY DEENERGIZING CIRCUIT TO MAKE NEW CONNECTION.
- JUNCTION BOX APPROXIMATELY 20' FROM DOOR. WIRING IS PULLED TIGHT THROUGH JUNCTION BOX. IMCC ELECTRICIAN WILL ASSIST WITH TEMPORARILY DE-ENERGIZING CIRCUIT TO MAKE NEW CONNECTION.
- WIRING HAS AN IN-LINE CONNECTOR AT CENTRAL BUILDING JUNCTION BOX APPROXIMATELY 150' FROM DOOR. IMCC ELECTRICIAN WILL ASSIST WITH IDENTIFICATION OF WIRES AT CENTRAL JUNCTION BOX.
- 9. MECHANICAL LOCK. CONNECT NEW DOOR POSITION SWITCH(S) TO EXISTING WIRING.



3

REPLACEMENT

APPROVED: RCC
ISSUED FOR: CONSTRUCTION
DATE: 03/13/2019
PROJECT NO4186070
FIELD BOOK:

NFORMATION

A600

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